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WINDFARM AT DEREENACRINNIG WEST,

DRUMOLEAGUE, CO. CORK

SITE INVESTIGATION CONTRACT

GEOTECHNICAL REPORT

NO. P16177

<u>Client</u>: Mr. George O'Mahony

Engineer: Jennings O'Donovan & Partners

Consulting Engineers,

Finisklin,

Sligo,

Ireland.



REPORT CONTROL SHEET

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Employer's Representative	Jennings O'Donovan & Partners Consulting Engineers					
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1 INTRODUCTION

1.1 SCOPE OF WORKS

In November 2016, Jennings O'Donovan & Partners, Consulting Engineers acting as the Employer's Representative on behalf of their Client, Mr. G O'Mahony commissioned Priority Geotechnical (PGL), to carry out a site investigation contract for the proposed windfarm development at Dereenacrinnig West, Drumoleague, Co. Cork. The windfarm development comprised of seven (7) turbines, 3km of associated access roadways, upgrade of 1.3km of existing roadway and electrical sub-station infrastructure.

The purpose of the investigation is to provide geotechnical information for the provision and design of the windfarm development and associated works.

The scope of the ground investigation, which was specified by Mott MacDonald, as Tendered (initially) comprised of the following:

- 32Nr. Trial pits excavations;
- 7Nr. Rotary core boreholes;
- 8Nr. Dynamic probes;
- Standpipe well installations;
- All associated sampling;
- In-situ testing, standard penetration testing and California bearing ratio (CBR);
- Geophysical investigation, MASW and electrical resistivity;
- Laboratory testing of soil and rock samples;
- Factual and interpretative reporting.

The final works as completed are detailed in Section 3.2 of this factual report. The fieldworks were carried out between 22nd November and 22nd December, 2016.

This investigation was carried out in accordance with the contract specification: Specification and Related Documents for Ground Investigation in Ireland (Engineers Ireland, October 2006), Eurocode 7- Geotechnical Design Part 2, ground investigation and testing (BS EN 1997-2: 2007) and the relevant British Standards (BS 5930 (1999) Code of Practice for Site Investigation +A2:2010 and BS 1377, Method of Tests for Soil for Civil Engineering Purposes, *in situ* Tests Parts 1 to 9).

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1.2 REPORTING

This geotechnical report (ref: P16177_RP_F01) presents the factual records of the fieldwork with respect to the site investigation works contract for the proposed windfarm development at Dereenacrinnig West, Drumoleague, Co. Cork and the geotechnical interpretation of same.

A non-intrusive geophysical survey was undertaken by PGL. A report titled "Dereenacrinnig Wind Farm, Geophysical Investigation" (ref: PGL P16177_GP_Rp_F01, *March, 2017*) is presented in **APPENDIX B** of this combined factual and interpretative report.

No responsibility can be held by PGL for ground conditions between exploratory locations. The exploratory logs provide for ground profiles and configuration of strata relevant to the investigation depths achieved during the fieldworks. Caution shall be taken when extrapolating between such exploratory locations. No liability is accepted for ground conditions extraneous to the exploratory locations. The geotechnical interpretation may be subject t change where further information becomes available.

No account has been taken of potential subsidence or ground movement due to mineral extraction, mining works or karstification below or in proximity to the site, unless specifically addressed.

This report has been prepared for the Employer and their Representative as outline, herein. The information should not be used without their prior written permission. PGL accepts no responsibility or liability for this document being used other than for the purposes for which it was intended.

1.3 SITE LOCATION & DESCRIPTION

The up-land site approx' 1km² was north of Castledonovan in Co. Cork, at Dereenacrinning West, at elevations 200mOD to 400mOD Malin.

1.4 PUBLISHED GEOLOGY

The published geology maps; Geological Society of Ireland, GSI, Sheet 24 1:100,000 mapping shows the site to be underlain the Gun Point Formation (GP) described as greengrey Sandstones and purple Siltstones. This formation has been observed to be well folded and generally steeply dipping 50° to 60° across the site. The south of the site is underlain by the Castlehaven Formation (CE), described as purple Mudstone and Siltstone. The very north of the site is underlain by the Toe Head Formation (TH) described as cross-bedded sandstones and purple Siltstones with some Mudstones. There are two published bedrock faults within the site area. Both faults run in a generally northeast-southwest direction, one passing under the northern boundary of the site and one passing close to the southern boundary. The Teagasc mapping on the GSI website shows this area of the site to be underlain by blanket Peat, glacial tills derived from Devonian Sandstones and shallow bedrock. Peat deposits (H3- H7) were measured across the site by Others (2010) and found to be between 0.1m to 3.15m deep.



2 FIELDWORK

2.1 GENERAL

The fieldwork was carried out in general accordance with British Standards (BS 5930 (1999) Code of Practice for Site Investigation +A2:2010 and BS 1377, Method of Tests for Soil for Civil Engineering Purposes, *in situ* Tests Parts 1 to 9). Details of the equipment and plant used are presented below.

Operation	Equipment	Nominal diameter,	Flush	Comments
		mm		
Rotary Coring and	Delta Base 520	Symmetrix 131mm	Compressed	Standard Penetration Test, N
Open Hole Drilling	6t tracked rig	diameter open hole	Air mist	values obtained in overburden.
				Visual observations of ground
		76mm diameter core		and groundwater conditions.
				Installation of standpipe
				monitoring wells.
Trial pit	12t tracked	N/A	N/A	Visual observations of ground
excavations	excavator			and groundwater conditions.
				Bulk disturbed sampling.
Dynamic probe TRL		8kg/ 575mm drop ht.	N/A	Pen rate mm/blow recorded &
				CBR estimated.
	Competitor	50kg/ 500mm		
	DP(H)			N _{100 H} blows/100mm

The exploratory locations were selected by JO'D and set out subject to work space restrictions and available access. The 'as constructed' exploratory locations were subsequently surveyed using Trimble V8 GPS equipment to the Ordinance Survey, Irish Transverse Mercator (ITM) system of co-ordinates and elevations to Malin Head datum. These locations are shown on the Exploration Location Plans (dwg. No.: P16177-SI-0A, P16177-SI-01 to P16177-SI-06) presented in **APPENDIX D** of this report.

2.2 EXPLORATORY HOLES

The exploratory holes as completed during the ground investigation are listed in the following table(s):

Туре	Quantity, Nr.	Depth Range, m bgl	Comments	
Trial Pit Excavations	32	0.55 – 3.20	TP01A ¹ , TP02A, TP03A, TP04A, TP05A,	
			TP06A and TP07A.	
			TP01, TP02, TP03, TP04, TP05, TP06,	
			TP07, TP08, TP09, TP10, TP11, TP12,	
			TP13, TP14, TP15, TP16, TP17, TP18,	
			TP19, TP20, TP21 and TP22.	
			TPS1, TPS2 and TPS3.	
Rotary Boreholes	4	6.70– 7.10	BHT01, BHT02, BHT03 and BHT04 ² .	
Dynamic probes	8	0.60 - 3.30	DP01, DP02, DP03, DP04, DP05, DP06,	
			DP07 and DP08 (existing road from	
			Castledonovan).	

SUMMARY OF EXPLORATORY HOLES

2.3 SAMPLING

Fifty six (56) bulk disturbed samples (B), forty nine (49) small disturbed samples (D), and 21.25lin.m rotary core were recovered from the exploratory holes in accordance with Geotechnical Investigation and Sampling – Sampling Methods and Groundwater Measurements (EN ISO 22475-1:2006).

¹ Annotation A referenced turbine locations; TP01A = T01 (BHT01).

² Rotary boreholes at turbines T05 – T07 were removed from the scope of works. Trial pits TP05A, TP06A and TP07A were excavated to bedrock. Details of bedrock is also provided for in the geophysical survey report, **APPENDIX B**.

2.4 GROUNDWATER MONITORING

Groundwater was recorded when encountered/ observed during boring and trial pit excavations over a period of 20 minutes, noting any changes that may occur. Groundwater levels were also monitored at start and end of drilling shifts. Groundwater is presented in a table in Section 5 of this factual report.

It should be noted that the normal rate of boring may not permit the recording of equilibrium groundwater levels for any one groundwater water strike where casing may exclude low volume flows as the borehole progresses. Groundwater conditions observed in the borings or pits are those appertaining to the period of the investigation. Groundwater levels may be subject to diurnal, seasonal and climatic variations and can also be affected by drainage conditions, tidal variations etc. The groundwater regime should be assessed from standpipe well installations, where available.



Arisings backfill to borehole



BENTONITE grout backfill to rotary boreholes/ installation

GRAVEL backfill to installation/ rotary borehole



uPVC slotted pipe

2.5 IN SITU TESTING

Standard Penetration Tests, N values, were typically carried out in the boreholes using the 60° solid cone (CPT) in place of the standard split barrel sampler. The Standard Penetration Test was carried out in accordance with Geotechnical Investigation and Testing, Part 3 Standard penetration test, BS EN ISO 22476-3:2005+A1:2011. The data was presented on the relevant logs in **APPENDIX A**.

The geophysical survey comprised of Resistivity Soundings, Seismic Refraction Profiling and Multi-Channel Analysis of Surface Wave (MASW) at 7 proposed turbine locations as well as a proposed substation location. The survey fieldwork was carried out by PGL between the 12th and 14th December, 2016. A separate report has been produced and is presented in **APPENDIX B** of this report.

TRL dynamic probes (8kg drop weight, 575mm drop height) were carried out in pit excavations to establish *in situ* California bearing ratio, CBR to refusal depths between 0.18m bgl to 2.30m bgl. The data from the testing was presented in **APPENDIX A** with the relevant trial pit records.

PGL's Competitor dynamic probing rig was to undertake dynamic probing (DP(H); heavy, 50kg drop weight, 500mm drop height) in general accordance with Geotechnical Investigation and Testing, Part 2, Dynamic probing, BS EN ISO 22476-2:2005. The blows per 100mm (N₁₀₀) were recorded to refusal being 25blows without progress over 100mm. Eight (8) number dynamic probes (DP01, DP02, DP03, DP04, DP05,DP06, DP07 and DP08) were advanced to refusal to depths between 0.6m below existing ground level (bgl) to 3.3m bgl.

Туре	Quantity	Remarks		
Standard penetration test,	4Nr.	Nspt refusals N presented a numerical value 50;		
N _{SPT} value		See APPENDIX A		
Geophysical survey	840lin.m 2D res	P16177_GP_Rp_F01, March, 2017		
	161lin.m Seismic/	See APPENDIX B		
	MASW			
Dynamic probing (TRL)	32Nr.	DCP01A, DCP02A, DCP03A, DCP04A, DCP05A,		
		DCP06A and DCP07A.		

SUMMARY OF IN-SITU TESTING

Туре	Quantity	Remarks
		DCP01, DCP02, DCP03, DCP04, DCP05, DCP06,
		DCP07, DCP08, DCP09, DCP10, DCP11, DCP12,
		DCP13, DCP14, DCP15, DCP16, DCP17, DCP18,
		DCP19, DCP20, DCP21 and DCP22.
		DCPS1, DCPS2 and DCPS3.
		See APPENDIX A
Dynamic probing (DPH)		N ₁₀₀ н
	8Nr.	See APPENDIX A

The distribution of N₁₀₀ values with depth (m below existing ground level, bgl) is presented below for dynamic probing DP(H). Refusals, N₁₀₀ =25, were plotted. The N₁₀₀ values for the boreholes are presented on probe logs in **APPENDIX A**.



3 LABORATORY TESTING

All samples were transported to Priority Geotechnical's laboratory in Midleton, Co. Cork examined, logged and prepared for scheduled testing. Laboratory testing was proposed by PGL, being revised and approved by Jennings O'Donovan. Testing was carried out by PGL, in accordance with BS1377 (1990), Methods of test for soils for civil engineering purposes and the ISRM suggested methods for rock characterisation, testing and monitoring. Specialist chemical testing was undertaken by Chemtest Ltd. (UK) on behalf of PGL. Specialist rock testing was carried out by GSTL Ltd (UK) on behalf of PGL. The laboratory test results were presented in **APPENDIX C**. A summary of tests undertaken were detailed below.

		301L3		
Туре	No.	Remarks		
Natural Moisture Content	39	9% to 795%		
Atterberg Limits	08	Liquid limit, LL 29% to 187%		
		Plastic limit, PL 23% to 102% and NP non-plastic soils		
		Plasticity index, PI 7 to 85		
Particle Size Distribution (grading)	15	See APPENDIX C		
рН	03	7.4 to 8.1		
SO4 water soluble	03	<0.010g/l to 0.013g/l		
SO4 acid soluble	03	<0.01%		
Loss on ignition	04	0.73% to 81%		
Proctor compaction (Moisture	01	TP06 0.75m;		
content/dry density relationship)		Optimum moisture content 6%		
		Maximum dry density 2.02Mg/m ³		
California Bearing Ratio, CBR	16	0.5% to 2.5%		
CBR Moisture content relationship	01	TP06 0.75m		
		See APPENDIX C		

SUMMARY OF LABORATORY TESTING UNDERTAKEN – SUPERFICIAL DEPOSITS

0000

60 CE 50 Plasticity Index, Pl CV ME 40 30 СН MV 0 20 CI CL MH 10 MI ML 0 10 20 30 40 50 60 70 80 90 100 0 Liquid Limit, LL %

Summary of plasticity data

SUMMARY OF LABORATORY TESTING UNDERTAKEN – SOLID GEOLOGY

ROCK				
Туре	No.	Remarks		
UCS with Young's Modulus and	01	16MPa		
Poisson's Ratio		55.7GPa		
		0.215		
Point Load Test (IP50)	06	0.7MPa to 4.7MPa		
Magnesium sulphate soundness	04	6% to 9%		
value MSSV,				
рН	01	7.4 to 8.7		
SO4 water soluble	01	<0.010g/l		

4 GROUND CONDITIONS

The full details of the ground conditions encountered are provided for on the exploratory records accompanying this report. The records provide descriptions, in accordance with BS 5930 (1999) +A2: 2010 and Eurocode 7, Geotechnical Investigation and Testing, Identification and classification of soils, Part 1, Identification and description (EN ISO 14688-1: 2002),– Identification and Classification of Soil, Part 2: Classification Principles (EN ISO 14688-2:2004) and Identification and Classification of Rock, Part 1: Identification & Description (EN ISO 14689-1:2004) of the materials encountered, in situ testing and details of the samples taken, together with any observations made during the site investigation.

The turbine sites were characterised by dark brown to black PEAT and peaty CLAY 0.22m to 0.6m thick and slightly sandy gravelly SILT with Cobble content deposits 0.7m thick to 1.1m thick overlying bedrock BHTP01 to BHTP04 (T01 – T04). Based on tactile assessment, the Silt was described as soft. Medium strong to strong SILTSTONE was encountered 1.55m bgl (BHT03) to 1.65m bgl (BHT01). Sandstone was found to underlie the Siltstone at BHT01 and BHT04, below 4.5m bgl to 5.05m bgl. Strong SANDSTONE was encountered at 1.55m bgl (BHT02). Peaty CLAY 0.8m thick was encountered at TP05A (T05) overlying 0.7m of firm slightly sandy gravelly SILT with Cobble content to a depth 1.5m bgl. Peaty CLAY 0.17m thick was encountered at TP06A (T06) overlying 0.63m of firm slightly sandy slightly gravelly SILT with Cobble content to a depth 0.8m bgl. PEAT 0.28m thick was encountered at TP07A (T07) overlying 0.92m of firm to stiff slightly sandy gravelly SILT with Cobble content to a depth 1.2m bgl.

The access road was characterised by mixed deposits. Topsoil; slightly sandy slightly gravelly SILT, 180mm to 300mm thick; PEAT 170mm to 600mm thick and peaty CLAY 150mm to 900mm overlay silty sandy GRAVEL (TP01, TP02, TP03 and TPS1) with varied Cobble content deposits 600mm to 1.55m thick and soft becoming firm to stiff, slightly sandy slightly gravelly SILT with varied Cobble content, 650mm to 2.7m thick. The Gravels and Silts were underlain by shallow bedrock 0.75m bgl to 2.3m bgl. Clayey PEAT was encountered 2.0m deep at TP12.

5 GROUNDWATER CONDITIONS

Groundwater was not encountered during rotary drilling. Groundwater was encountered in trial excavations between ground level, 0.00m bgl and 2.00m bgl. Details of the ground water and installations are presented graphically on the relevant exploratory logs within **APPENDIX A** and are summarised below. See also section 3.4 for general details.

No standpipe well, installations were constructed.

SUMMARY OF GROUNDWATER INFORMATION OBTAINED DURING SITE WORKS

Location	Depth water	Commente			
Location	strike, m bgi	Comments			
TPS1	2.00	Water strike at 2.0m bgl, trickle flow rate.			
TP01	0.80	Water strike at 0.80m bgl. Trickle flow rate.			
TP05A	0.30	Trickle flow rate.			
TP07	0.10	Water strike at 0.10m bgl. Steady flow rate.			
TP09	1.40	Water strike at 1.40m. Trickle flow rate.			
TP12	0.10	Water strike at 0.10m bgl. Trickle flowrate.			
TP20	1.20	Waterstrike at 1.20m bgl. Trickle flow rate.			
TP21	0.00	Water encountered at ground level.			

6 GEOTECHNICAL REVIEW

6.1 GROUND MODEL

The site was characterised by shallow bedrock overlain by organic (shallow blanket PEAT) and mixed glacial deposits slightly sandy slightly gravelly SILT and silty sandy GRAVEL.

Groundwater was assumed at the boundary of the rock and superficial deposits with perched groundwater within the shallow peat deposits.

6.2 CHARACTERISTIC PROPERTIES

6.2.1 PEAT

The Peat was characterised by extremely high plasticity (liquid limit, LL 187%), high organic content 81% and natural moisture content, *w* 248% and 765%.



Summary of plasticity data

The Peat deposits are expected to be highly compressible, noting Peat was 2m thick at location TP12.

Historical in situ hand vane tests (Others, 2010) indicated undrained shear strengths 16kPa to 34kPa describing very soft to soft deposits.

6.2.2 SILT

The Silt was characterised by natural moisture content, w 10% to 45%, low plasticity (ML) and low organic content (<6%). Grading analysis indicated 22% to 42% silt fraction with 29% to 52% gravel fraction, 5% to 34% sand fraction with up to 20% coarse Cobble content. The deposits are described as mixed glacial tills.



Summary of plasticity data

Moisture content profile - Silt



Moisture content, %

The ratio of natural moisture content to plastic limit indicated soft (*w*/PL >1.2) to 'firm to stiff' deposits (*w*/PL <1.0) (C504 Engineering in glacial tills, Figure 5.19). It can be seen that soft deposits were present to depths up to 1.3m bgl at locations; *TP04*, TP05, TP06, *TP07*, TP11, TP13, TP15, TP17, TP18, *TP19*, TP03A and *TPS3*. Undrained shear stenghts are expected to vary between 20kPa and >75kPa ('very soft to soft' to stiff, BS5930; 1999). The soft deposits were typically in the upper 0.5 -1.0 with exceptions of TP04, TP07, TP19 and TPS3 where the soft deposits extended deeper. It is noted the *in situ* tactile assessment indicated firm deposits (40kPa – 75kPa BS5930; 1999). Plasticity data indicated a friction angle, $\phi = 26^{\circ}$ (C504 figure 5.1).



Strength profile

6.2.3 GRAVEL

The Gravel was characterised by natural moisture content, w 10% to 42% and intermediate to very high plasticity (MI – MV). Grading analysis indicated 34% to 66% gravel fraction with 14% to 25% sand fraction, 7% to 27% silt fraction with 4% to 24% coarse Cobble content. The deposits are described as mixed glacial tills.



Summary of plasticity data

Moisture content profile - Gravel



Moisture content, %

The ratio of natural moisture content to plastic limit indicatd soft (w/PL >1.2) to 'firm to stiff' deposits (w/PL <1.0). It can be seen that soft deposits were present to depths up to 0.8m bgl at locations; TP08 and TPS3.



No assessment of the relative density of the Gravels was provided. The higher plasticity of the Gravels were atributed to the overyling organic deposits.

The SILT and GRAVEL shall be considered a singular deposit - mixed glacial till with varied granular fractions, having similar characteristics with only variation in the propositions of the varied constituents/ fractions and characterisd by varied undrained shear strengths.

6.2.4 SANDSTONE and SILTSTONE

The rock mass characterization has been established using the Rock Quality Designation, (RQD, Deere, 1964), Rock Mass Rating (RMR) using the Geo-mechanics System (Bieniawski, 1989) and Geologic Strength Index (GSI, Hoek and Brown 1997, 2002). A review of the rock properties, strength (medium strong to strong IP50, 1.1MPa – 4.7MPa, UCS 16MPa), fracture spacing (NI– 400mm) and condition (slightly weathered no infill), Rock Quality Designation (RQD non-intact/ 9% - 78%) and groundwater (assumed 'dry' within the zone of influence of the expected foundations) was undertaken. The rock mass rating, RMR range was 44 - 64.

The Siltstone and Sandstone bedrock was classified as Class III-II fair to good rock with unit weight 2.71Mg/m³. A Poisson's ratio, υ of 0.15 - 0.20 can be expected and was measured as υ = 0.215. The intact Young's Modulus, E was 55.7GPa. Allowing for the non-intact nature of the rock mass this was reduced by a factor of 0.15 yielding a design modulus of 8.4GPa.

	T01		T02		т		03		Т04	
RMR	44	52	62	64	59	64	54	63	54	57
Class			П		-		111-11		111	
Cohesion	250kPa		300kPa		300kPa		275kPa		250kPa	
Friction	30°		35 °		35 °		32.0		30 °	
Rock	Siltstone		Sandstone		Siltstone		Sandstone		Siltstone	
E ³ , MPa	5093	9530	18430	20761	15303	20761	10979	19570	10979	13446
	7079	11220	19953	22387	16788	22387	12589	21135	12589	14962

The rock characteristic properties were summarized as follows:

Note there was a reasonable correlation between the Young's Modulus, E derived from direct measurement (BHT03) the assessment of rock mass rating, RMR and the indirect geophysical measurements (MASW).

6.3 PAVEMENT AND HARDSTANDING CONSTRUCTION

It is recommended to construct pavement and hardstanding from a formation level in weathered bedrock were practical. Where the depth of superficial deposits exceeds 1.0m 600mm capping is recommended. Laboratory test data and moisture content data suggested an adjustment factor of 0.5 for the *in situ* determined CBR values. CBR values of 22% were attributed to coarse particles or transition to the weathered rock mass. It is recommended that plate loading tests, using a 600mm plate, be used to control CBR during the construction phase for both the roadway and also hardstanding associate with crane platforms. The risk associated with pavement failure are high during construction and commissioning phased of windfarm development.

 $^{^{3}}$ E =3.5 x RMR^{3.75}/1000 and 10ⁿ where n = (RMR-10)/40 (*Boyd*, 1993; BS8004)

Natural moisture content data and a single California bearing ratio, CBR/ moisture content relationship, suggested CBR2% at moisture content, w > 10%. Typically the superficial deposits were wet of optimum moisture content (6%/ CBR50%) and so low CBR values were measured. It is not seen as practical to 'dry' the deposits *in situ* and so a design capping thickness of 600mm is proposed within the sandy gravelly SILT and silty sandy GRAVEL deposits with 150mm sub-base (cl.804 or similar) unbound surfacing. A summary assessment of CBR data is presented in **APPENDIX E**.

Where the sub-formation, the underside of capping (subgrade) is on weathered rock, no capping is required and 150mm sub-base (cl.804 or similar) shall be provided subject to a review by the Engineer.

At natural moisture content <15%, compaction levels of 95% maximum dry density can be achieved. It is recommended that the sub- grade deposits (slightly) sandy (slightly) gravelly SILT, be compacted to increase stiffness. Between moisture content 5% to 19%, CBR >20% is expected.

'Reversal road' construction shall be considered where the weathered rock mass was at depths <1.0m bgl. Shallow road cuttings are also recommended. A minimum road width at formation of 7m is expected in shallow cutting and through Peat deposits.

Drainage shall be provided a minimum of 450mm below sub-formation.

6.3.1 Existing roadway

Dynamic probing DPH indicated varied strength to depths between 0.6m bgl and 3.3m bgl. Undrained shear strength was between 20kPa and 100kPa indicting soft deposit typically becoming firm to stiff with depth below 1.0m bgl.

California bearing ratio, CBR was derived based on the relationship;

CBR (%) =
$$s_u (kPa) / 15$$
.

It can be seen that CBR2% is expected to varied depths between 1.0m bgl and 2.0m bgl.

In order to fully assess the existing pavement the full construction detail, sub-base and surfacing thicknesses should be reviewed.

A pavement condition survey and Falling Weight Deflectometer FWD analysis should be undertaken to assess the structural integrity of the existing roadway to take the temporary trafficking associated with turbine deliveries.







6.4 RE-USE OF AGGREGATE MATERIALS

With a 10% fines value of 102kN to 117kN and magnesium sulphate soundness values, MSSV 6% to 9%, the Sandstone and Siltstone is expected to be suitable for re-use as capping with 50kN < 10%FV < 130kN and MSSV <25%.

No direct assessment was made of borrow pits and so further assessment is recommended where aggregate is produced on site. Water absorption capacity WAC (limit <2%) is recommended along with aggregate impact value AIV (limit <35%, soaked) to assess strength and durability. Flakiness index (FI <40), grading (<6% fines content) and plasticity (Liquid limit <21%) of aggregate shall also be assessed.

6.5 FOUNDATIONS

Foundations for the proposed turbines are within the range 13.2m to 16.8m outer diameter (da) and 1.6m thick (hges);



Minimum allowable bearing pressure of 77kPa to 127kPa is required for settlements not exceeding 3mm/m over a period of 20 years.

6.5.1 Turbine sites

Turbines are expected to be founded within the bedrock at least below 1.7m bgl. Based on the core information for the upper 5.5m of bedrock the rock was described as Class 3, weakly cemented Sandstone/ Siltstone ⁴ in accordance with Figure 4. Weak and broken rock of the Code of practice for Foundations, BS8004, 1986. An allowable bearing capacity of 250kPa is provided for settlement up to 0.5% foundation width (Figure 1, BS8004) where the bedrock is non-intact in the upper zone weathered zone.

For settlement of the order 3mm/m an allowable bearing pressure of 130kPa to 255kPa is recommended taking a closely spaced, fracture spacing, *j* 100mm to 400mm and a single unconfined compressive strength, q_c of 16MPa ³. The geophysical survey data indicated an increasing Modulus with depths and so the proposed allowable bearing pressure is likely to be conservative over the full depth of influence of the foundations and where the rock strength is stronger. Shear failure of the foundation is not expected at the level of loading applied where an ultimate bearing pressure of 4MPa is expected. Settlement, particularly differential settlement, will be the limiting factor.



⁴ A UCS of 16MPa was measured conservatively indicating a weak rock in contrast with high point load indices up to 4.7MPa indicating strong rock.

Turbine; T01 foundations are recommended to be constructed below 371mOD within the bedrock. Bases on the rock mass characteristic at the turbine location a lower limit of allowable bearing pressure of 160kPa is recommended.



Seismic profile T02

Turbine; T02 foundations are recommended to be constructed below 393mOD within the bedrock. Bases on the rock mass characteristic at the turbine location an allowable bearing pressure of 255kPa is recommended.



Turbine; T03 foundations are recommended to be constructed below 396mOD within the bedrock. Bases on the rock mass characteristic at the turbine location an allowable bearing pressure of 215kPa is recommended.



Seismic profile T04

Turbine; T04 foundations are recommended to be constructed below 379.5mOD within the bedrock. Note the rotary borehole appears to be at a location at a higher elevation. Bases on the rock mass characteristic at the turbine location an allowable bearing pressure of 130kPa is recommended.



Turbine; T05 foundations are recommended to be constructed below 359mOD within the bedrock. Bases on the rock mass characteristic derived from geophysical investigation at the turbine location an allowable bearing pressure of 130kPa is recommended.



Seismic profile T06

Turbine; T06 foundations are recommended to be constructed below 373mOD within the bedrock. Bases on the rock mass characteristic derived from geophysical investigation at the turbine location an allowable bearing pressure of 195kPa is recommended.



Turbine; T07 foundations are recommended to be constructed below 347mOD within the bedrock. Bases on the rock mass characteristic derived from geophysical investigation at the turbine location an allowable bearing pressure of 215kPa is recommended.

In all cases T01 - T07, the recommended lower limit of allowable bearing pressure exceeds the minimum requirement for the proposed foundation design where the foundations are constructed within the rock mass.

It is recommended at construction phase that plate bearing tests are carried out to verify the bearing pressure and assess the Modulus of the rock mass at foundation level.

6.5.1.1 Excavation

Excavation where required shall be by means of hydraulic excavator with hydraulic ripping and some hydraulic breaking in the upper zone of the slightly weathered rock. The energy requirement has not been assessed but can be expected to vary with location and depth. The rates of excavation at turbines; T02 and T03 will be reduced where fracture spacing and RQD was greater.



6.5.2 Substation

Silty sandy GRAVEL/ slightly sandy gravelly SILT was identified below a depth 0.55m bgl to 1.1m bgl TRL probing indicated CBR22% indicative of medium dense granular deposits or firm to stiff cohesive deposits.

TPS3 at a depth 1.1m bgl yielded a w/PL = 1 indicative of undrained shear strength 75kPa (C504 figure 5.19). An ultimate bearing pressure of 385kPa is expected. This should be verified by direct measurement using plate loading tests to determine soil strength. A safe bearing pressure of 75kPa is recommended (BS8004; Code of practice for Foundations; 1986) for settlements up to 25mm below a depth 1.0m bgl where the Silt fraction (17% to 44%) is considered dominant in terms of soil behaviour. Traditional shallow foundations will be suitable for the sub-station structure.

6.6 GROUNDWATER

Control of groundwater will be required during road construction. Appropriate drainage shall be provided to keep groundwater below formation level. Perched groundwater flow can be expected within the peaty deposits over the Silt.

6.7 CHEMICAL

pH and sulphate data indicated the site to be characterised by design sulphate class DS-1 in accordance with BRE special digest 1 (2003) concrete in aggressive ground or XA1 in accordance with IS EN 206.

7 REGISTER OF GEOTECHNICAL HAZARDS

Geotechnical hazards comprise of;

- Poor formation in the existing roadway;
- Localised soft extremely high plasticity compressible organic deposits, excessive rutting, long term settlement;
- Perched groundwater flow;
- Localised shallow bedrock, excavation in rock and
- Varied weathering profile of the rock mass.

The exact extents associated with the geo-hazards identified are not fully defined specifically the deeper Peat deposit associate with TP12.

8 SUMMARY

- The turbine sites were characterised by dark brown to black PEAT, peaty CLAY and slightly sandy (slightly) gravelly SILT with varied Cobble content deposits 1.55m thick to 1.65m thick overlying bedrock. Medium strong to strong SILTSTONE was encountered 1.55m bgl (T03) to 1.65m bgl (T01). Sandstone was found to underlie the Siltstone at T01 and T04, below 4.5m bgl to 5.05m bgl. Strong SANDSTONE was encountered at 1.55m bgl (BHT02). Firm to stiff slightly sandy gravelly SILT overlay Bedrock 0.8m to 1.5m bgl at turbines; T05 to T07.
- 2. The access road was characterised by mixed deposits. Topsoil (slightly sandy slightly gravelly SILT/ clayey PEAT and PEAT, 150mm to 900mm thick, overlay silty sandy with varied Cobble content deposits 600mm to 1.55m thick and soft becoming firm to stiff, slightly sandy slightly gravelly SILT with varied Cobble content, 650mm to 2.7m thick. The Gravels and Silts were underlain by shallow bedrock 0.75m bgl to 2.3m bgl.
- 3. Clayey PEAT was encountered 2.0m deep at location TP12.
- 4. TRL dynamic probing was carried out at the thirty two (32) number trial pit locations providing for an estimate of unadjusted California bearing ratio, CBR. The data is presented in **APPENDIX A** of this report.
- 5. Dynamic probing DPH was carried out at eight (8) number locations along the existing access roadway from Castledonovan. The data is presented in **APPENDIX A** of this report.
- 6. A non-intrusive geophysical survey utilising Resistivity Soundings, Seismic Refraction Profiling and Multi-Channel Analysis of Surface Wave (MASW) at 7 proposed turbine locations techniques were carried out by PGL. A report ref: P16177_GP_Rp_F01, *March, 2017, is* presented within **APPENDIX B** of this report.
- Groundwater was encountered between depths of 0.0 m bgl (ground level) and 2.0m bgl in trial pit excavations. Details are summarised herein and presented on the relevant trial pit logs presented in APPENDIX A of this report.
- 8. No groundwater was encountered during rotary drilling at the turbine locations. No standpipe wells were installed, see Section 5.
- 9. Perched groundwater flow can be expected within the peaty deposits over the Silt.
- 10. Detailed records of the ground and groundwater conditions can be found on the exploratory logs and photographic records presented within **APPENDIX A** of this report. Further details of the ground conditions can be found in the geophysical survey report presented within **APPENDIX B** of this report.
- 11. Laboratory testing was undertaken to determine the classification, engineering properties of the soil and rock encountered during the site investigation. The data is presented in **APPENDIX C** of this report.
- 12. The exploratory locations are presented on the location plans presented within **APPENDIX D** of this report.
- 13. The Peat (CE) deposits are expected to be highly compressible. Historical in situ hand vane tests (*Others, 2010*) indicated undrained shear strengths 16kPa to 34kPa describing very soft to soft deposits.
- 14. The Silt (ML) was described as soft to 'firm to stiff' deposits. The soft deposits were present to depths up to 1.3m bgl at locations; *TP04*, TP05, TP06, *TP07*, TP11, TP13, TP15, TP17, TP18, *TP19*, TP03A and *TPS3*. Undrained shear stenghts are expected to vary between 20kPa and >75kPa. Plasticity data indicated a friction angle, $\phi = 26^{\circ}$.
- 15. The SILT and GRAVEL shall be considered a singular deposit mixed glacial till.
- 16. The rock mass rating, RMR range was 44 64. The Siltstone and Sandstone bedrock was classified as Class III-II fair to good rock with unit weight 2.71Mg/m³. A Poisson's ratio, υ was measured = 0.215. The intact Young's Modulus, E was 55.7GPa. Allowing for the non-intact nature of the rock mass this was reduced by a factor of 0.15 yielding a design modulus of 8.4GPa. A friction angle between 30° and 35° with cohesion 250kPa to 300kPa are recommended.
- 17. The Sandstone and Siltstone is expected to be suitable for re-use as capping with 50kN
 < 10%FV < 130kN and MSSV <25%.

- 18. For settlement of the order 3mm/m at the proposed turbine locations, an allowable bearing pressure of 130kPa to 255kPa is recommended.
- 19. A safe bearing pressure of 75kPa is recommended below a depth 1.0m bgl for foundations associated with the sub-station infrastructure.
- 20. Excavation shall be by means of hydraulic excavator with hydraulic ripping and some hydraulic breaking in the upper zone of the slightly weathered rock. The rates of excavation at turbines; T02 and T03 will be reduced where fracture spacing and RQD was greater.
- 21. A capping requirement of 600mm is recommended for pavement design. No capping is required where the subgrade in on weathered rock, 150mm of surfacing cl. 804 subbase or similar is recommended. A summary of the CBR assessment is provided in **APPENDIX E**.
- 22. 'Reversal road' construction shall be considered where the weathered rock mass was at depths <1.0m bgl. Shallow road cuttings are also recommended. A minimum road width at formation of 7m is expected in shallow cutting and through Peat deposits. Drainage shall be provided a minimum of 450mm below sub-formation, the underside of capping
- 23. A CBR2% is expected to varied depths between 1.0m bgl and 2.0m bgl along the existing roadway as assessed by dynamic probes DP(H); DP01 to DP08.
- 24. The site was characterised by design sulphate class DS-1 in accordance with BRE special digest 1 (2003) concrete in aggressive ground or XA1 in accordance with IS EN 206.
- 25. A summary register of geotechnical hazards has been provided.
- 26. A number of recommendations are made with regard to quality control and verifying design details at construction phase, typically requiring *in situ* plate loading testing.

APPENDIX A

EXPLORATORY HOLE AND PHOTOGRAPHIC RECORDS

Rotary boreholes	BHT01, BHT02, BHT03 and BHT04 ⁵ .
Trial pit excavations/	TP01A, TP02A TP03A, TP04A, TP05A, TP06A and TP07A.
TRL dynamic probes	TP01, TP02, TP03, TP04, TP05 ⁶ ,TP06, TP07, TP08, TP09, TP10, TP11, TP12, TP13, TP14, TP15, TP16, TP17, TP18, TP19, TP20, TP21 and TP22.
	TPS1, TPS2 and TPS3.
	DCP01A, DCP02A, DCP03A, DCP04A, DCP05A, DCP06A and DCP07A.
	DCP01, DCP02, DCP03, DCP04, DCP05, DCP06, DCP07, DCP08, DCP09, DCP10, DCP11, DCP12, DCP13, DCP14, DCP15, DCP16, DCP17, DCP18, DCP19, DCP20, DCP21 and DCP22.
	DCPS1, DCPS2 and DCPS3.
Dynamic probes, DP(H)	DP01, DP02, DP03, DP04, DP05, DP06, DP07 and DP08.

 $^{^{5}}$ Rotary boreholes at turbines T05 – T07 were removed from the scope of works.

⁶ No photographic record is available for TP05.

KEY TO SYMBOLS ON EXPLORATORY HOLE RECORDS

All linear dimensions are in metres or millimetres

DESCRIPTIONS

**	Drillers Description
Friable	Easily crumbled
SAMPLES	
U()	Undisturbed 102mm diameter sample, () denotes number of blows to drive sampler
U()F, U()P	F- not recovered, P-partially recovered
U38	Undisturbed 38mm diameter sample
P(F), (P)	Piston sample - disturbed
В	Bulk sample - disturbed
D	Jar Sample - disturbed
W	Water Sample
CBR	California Bearing Ratio mould sample
ES	Chemical Sample for Contamination Analysis
SPTLS	Standard Penetration Test S lump sample from split sampler
CORE RECOVERY AND	ROCK QUALITY
TCR	Total Core Recovery (% of Core Run)
SCR	Solid Core Recovery (length of core having at least one full diameter as % of core run)
RQD	Rock Quality Designation (length of solid core greater than 100mm as % of core run)
Where there is insuffi	cient space for the TCR, SCR and RQD, the results may be found in the remarks column
lf	Fracture Spacing in mm (Minimum/Average/Maximum) NI - non intact, NR - no recovery
AZCL	Assumed Zone of Core Loss
NI	Non intact
GROUNDWATER	
\bigtriangledown	Groundwater strike
V	Groundwater level after standing period
Date/Water	Date of shift (day/month)/Depth to water at end of previous shift shown above the date and depth to water at beginning of shift given below the date
INSITU TESTING	
S	Standard Penetration Test - split barrel sampler
С	Standard Penetration Test - solid 60° cone
SW	Self Weight Penetration
lvp, HVp (R)	In Situ Vane Test, Hand Vane Test (R) demonstrates remoulded strength
K(F), (C), (R), (P)	Permeability Test
НР	Hand Penetrometer Test
MEASURED PROPERT	IES
Ν	Standard Penetration Test - blows required to drive 300mm after seating drive
x/y	Denotes x blows for y mm within the Standard Penetration Test
x*/y	Denotes x blows for y mm within the seating drive

c_u Undrained Shear Strength (kN/m²)

CBR California Bearing Ratio

ROTARY DRILLING SIZES

Index Letter	Nominal Diameter (mm)						
	Borehole	Core					
Ν	75	54					
н	99	76					
Р	120	92					
S	146	113					



Key Sheet

Pglpriority geotechnical							v Geotechnic 1 4631600 11 4638690 rioritygeotec	Borehole No. BHT01 Shoot 1 of 1						
Project Name: Dereenacrinnig Windfarm					Projec	ct No.		Co-ords:	510906E - 55 [°]	1802N	Hole Typ	f 1 De		
					P161	//		Level:	375.64m OD		RC Scale			
Client		Jenning	ns O'Donn	ovan 8	& Part	ners			Dates:	28/11/2016		1:50 Logged E	Ву	
	Water	Depth	Туре	Co	orina	(%)	Depth (m)		1	20,11,2010		JMS		
Well	Strike (m)	(m)	/Fs (min, max)	TCR	SCR	RQD	/ FI (/m)	(mOE) Legend	Str	atum Descripti	ion		
		50 (0 for 100mm/50 for 0mm) 1.6€€2.65 2.65 - 3.40 3.40 - 4.15 4.15 - 5.35 5.35 - 6.45 6.45 - 7.10	30mm 160mm 90mm	85 100 100 100	85 91 93 90 100	0 21 20 0 0	1.65 26/m 7.10	373.9	9 9 9 9 10. a shic, shi	Lithology: Moc SILTSTONE. Weathering: S apparent loss colouration ald smearing on fi Fractures: 2 s 0-10 degrees rough to smoo close spacing degrees with p fracture surfact	derately weak, g lightly weathere of strength, min ong fractures an racture surfaces ets apparent. Se with planar to un th fracture surfa Set 2 is dipping planar to undula ses and close sp	ad with or oxidation d minor clay s. et 1 is dipping hdulating aces and g 20-30 ting smooth bacing.		
Groun	dwater	se to After ~	nin Soolo	4	Com	ment	Hole In Hole D	formati epth (m)	on: Hole Dia (m	Im) Casing Dia	Chiselling:	Base Duration	Tool	
Suuch	x, 111 K	USE IU AILEI, II		No	ne enc	ountered	i. 7.	.10	Deltabasa	· (mm)		Darauori	.00	
Remar Boreh	ks: ole teri	minated at 7.2	10m bgl.				-4aihii		Shift Da	ata: Groundwater 28 Dry 28 Dry 29 Dry 29	Shift H 5/11/2016 08:00 5/11/2016 18:00 5/11/2016 08:00 5/11/2016 18:00	lole Depth (m) Re 0.00 Star 3.40 Enc 3.40 Star 7.70 E bo	emarks rt of shfit. d of shift. rt of shift. End of prehole.	









					Priority	/ Geotech	nnical Ltd.	Trial Pit No	
pgl	priority				l: 021 463 x: 021 46:	31600 38690	TP01A		
	geoteenneur				www.pri	oritygeot	echnical.ie	Sheet 1 of 1	
Projec Name	ct Dereenad	rinnig Wir	ndfarm	Pro P16	ject No. 177		Co-ords:510882.03 - 551794.23 Level: 372.69	Date 22/12/2016	
Locati	on: Drimolea	gue, Co. (Cork.	I			Dimensions (m): 3.60	Scale	
Client	: Jennings	O'Donno\	van & Partners				Depth:	Logged VT	
r e	Sam	ples & In Situ	u Testing	Denth	Level				
Wat Stril	Depth	Туре	Results	(m)	(m)	Legend	Stratum Description		
Wa	Depth 0.40 - 1.05 0.40 - 1.05 1.05 - 1.25 1.05 - 1.25	Type B D B D	Results	(m) 0.20 0.40 1.05 1.25	(m) 372.49 372.29 371.64 371.44	Legend	Soft, black, slightly gravelly peaty CLAY (Topsoil). Firm, light brown, slightly sandy gravelly SILT. San fire to coarse. Gravel is fine to coarse, angular to si- rounded. Sandstone lithology. Soft to firm, red brown, slightly gravelly slightly sar SILT with medium cobble content and low boulder content. Gravel is fine to coarse. Angular to sub- rounded. Sand is fine to coarse. Cobbles are angu sub-rounded, 63-200mm dia. Sandstone lithology. Soft, grey brown, slightly sandy gravelly SILT with cobble content and high boulder content. Cobbles angular to sub-angular, 63-200mm dia. Sandstone lithology. Boulders are angular to sub-angular, 200-500mm Sandstone lithology. End of Pit at 1.25m	d is sub- idy id id idy id idy id idy	
Stability Plant:	y: Good. 12t track mac	chine.				Groundy	vater: None encountered.	5 -	
Backfill Remark	: Arisings. (S: Trial pit term	inated at 1.2	25m bgl, bedrock.			<u> </u>			

DCP01A in TP01A

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	0	-	0.00	-
1	32	320	0.32	0
2	36	40	0.36	4
3	37	10	0.37	22
4	38	10	0.38	22
5	39	10	0.39	22
6	39	0	0.39	
7	39	0	0.39	
8	39	0	0.39	
9	41	20	0.41	9
10	43	20	0.43	9
11	44	10	0.44	22
12	45	10	0.45	22
13	45	0	0.45	
14	45	0	0.45	
15	46	10	0.46	22
16	46	0	0.46	
17	47	10	0.47	22

10

Unaulusieu CDN, /d	Unadi	iusted	CBR.	%
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 22
 CBR, Kleyn

 26
 CBR, TRL

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed

Photographic Record









						Priority Geotechnical Ltd. Tel: 021 4631600							Borehole No.	
pgipriority						Fax: 021 4638690 www.prioritygeotechnical.ie							BHT02	
	geoteenin						,,						Sheet 1 o	f 1
Projec	ct Name	: Dereer	nacrinnig V	Vindfa	rm	Projec P1617	ct No. 77		Co-	ords:	511129E - 551	861N	Hole Typ RC	be
Locati	ion:	Drimole	eague, Co.	Cork.					Lev	el:	395.70m OD		Scale 1:50	
Client	:	Jenning	gs O'Donn	ovan &	& Part	ners			Date	es:	25/11/2016		Logged E JMS	Зу
Well	Water Strike (m)	Depth (m)	Type /Fs (min, max)		oring	(%)	Depth (m) / FI (/m)	Leve (mO[) 2)	Legend	Stra	atum Descripti	on	
	(m)	50 (36 for Qnage)/50 for (C) 1.95 - 2.45 2.45 - 3.15 3.15 - 5.10 5.10 - 5.75 5.75 - 7.00	30mm 400mm	75 100 97 95 100 100 100	0 76 97 95 100	RQD 0 26 57 47 25 12	1.55 31/m 7.00	394.1	5 0		Open hole bor fine grained S/ Weathering: S loss of strengti colouration an fracture surface Fractures: 1 se with planar to s surfaces and c 1.55 - 1.95m: No	erately strong, (ANDSTONE. ightly weathere n, minor orange d minor clay sm es. et dipping 15-30 stepped rough f lose spacing. <u>n intact</u> .	ribed: Peat. grey purple, d with minor oxidation learing along degrees racture	
Groun Struck	dwater:	ose to After, n	nin Sealec		Com	ment	Hole In Hole De	formati	ion: Ha	ole Dia (mi	m) Casing Dia (mm)	Chiselling: Depth Top Depth	Base Duration	Tool
				No	ne enc	ountered	I. Equipm	nent:	De	eltabase	520			
Remar	ks:									Shift Da	Groundwater	Shift H	lole Depth (m) Re	emarks
Boreh	ole tern	ninated at 7.0	00m bgl.								25 Dry 25 Dry 28 Dry 28	/11/2016 08:00 /11/2016 08:00 /11/2016 08:00 /11/2016 18:00	2.75 End 2.75 Star 7.00 E bo	t of shift. t of shift. t of shift. End of rehole.





Priority Geotechnical Ltd.							Trial Pit	No		
pgi	PU priority geotechnical Fax: 021 4631600								TP02A	
					www.pri	oritygeot	echnical.ie	Sheet 1	of 1	
Projec	Dereenad	rinnig Wi	ndfarm	Pro	ject No.		Co-ords: 511125.00 - 551856.90	Date)	
Name	•			P16	177		Level: 396.25	22/12/20 Scale	016 •	
Locati	.ocation: Drimoleague, Co. Cork.					Dimensions (m):	1:25			
Client	: Jennings	O'Donno	van & Partners				Depth:	Logge	d	
ter ke	Sam	oles & In Sit	u Testing	Depth	Level					
Wa Stri	Depth	Туре	Results	(m)	(m)	Legend	Stratum Description			
						-110,	Soft, dark brown black, slightly gravelly peaty CL Gravel is fine to coarse, angular to rounded. (Top	.AY. osoil)	-	
	0.20 - 0.70 0.20 - 0.70	B		0.20	396.05		Soft, grey brown, slightly gravelly slightly sandy	SILT with		
	0.20 0.70						high cobble content and medium boulder conten is fine to coarse. Gravel is fine to coarse, angula	t. Sand r to sub-		
						****** *****	rounded, Sandstone lithology. Cobbles are angu sub-angular, 63-200mm dia. Sandstone lithology	lar to		
				0.70	395 55	× × × × ×	Boulders are angular to sub-angular, 200-500mr Sandstone lithology	n dia.		
				0.10	000.00		End of Pit at 0.70m	/	1 -	
									-	
									-	
									2	
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									-	
									-	
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									3 -	
									-	
									-	
									4 -	
									-	
									-	
									-	
									-	
									-	
									5 -	
Stabilit	y: Moderate.					Groundy	vater: None encountered.			
Plant: Backfill	I: Arisings.									
Remark	s: Borehole ter	minated at (0.70m bgl, bedrock.							

DCP02A in TP02A

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	0	-	0.00	-
1	20	200	0.2	0
2	21	10	0.21	22
3	21	0	0.21	
4	22	10	0.22	22
5	24	20	0.24	9
6	25	10	0.25	22
7	25	0	0.25	
8	26	10	0.26	22
9	27	10	0.27	22
10	27	0	0.27	
11	28	10	0.28	22
12	29	10	0.29	22
13	29	0	0.29	
14	29	0	0.29	
15	29	0	0.29	

10

Una	djusted	CBR, %
22	CDD	Kloup

22	сы, кіеуі
26	CBR, TRL

Adjustments

0.71moderate0.5dry0.35very dry0.5not assessed





Number:

TP02A

Project Project No Engineer

Derreenacrinnig Wind Farm P16177 Jennings O'Donovan & Partners





						Priority Tel: 021	Geotechnie 4631600	cal Ltd.			Borehole N	√ 0.
pg	Periority						1 4638690 rioritygeote	chnical.ie			BHT03	3
	5										Sheet 1 of	i 1
Projec	t Name	e: Dereer	nacrinnig V	Vindfa	rm	Projec P1617	ct No. 77	C	Co-ords:	511356E - 551903N	Hole Typ RC	e
Locati	on:	Drimole	eague, Co.	. Cork.				L	evel:	398.85m OD	Scale 1:50	
Client	Client: Jennings O'Donnovan & Part							C	Dates:	25/11/2016	Logged B JMS	3y
Well	Water Strike	Depth	Type /Fs (min,	Co	oring	(%)	Depth (m) / FI (/m)	Level	Legend	Stratum Descrip	otion	
	(m)	(m)	max)	TCR	SCR	RQD	()	(mOD) alte alte alte	Open hole boring. Driller des	scribed: Peat.	
									ઝીરંદ ઝીરંદ ઝીરંદ ૬ ઝીરંદ ઝીરંદ જ ઝીરંદ ઝીરંદ ઝીરંદ			
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		50 (49 for	-				1.55	397.30	k stk stk s <u>stk stk stk</u> ×××××××	Lithology: Moderately strong	purple red	
		75mm/50 for 0mm)								SILTSTONE.	, paipie : ca	2 -
		1.55 - 2.75	30mm 80mm	100	92	0	10/m			Weathering: Slightly weathe loss of strength and minor of oxidation on select surfaces	ed with minor ange	
	·		-							Fractures: 2 apparent sets. 30 to 40 degrees with plana	Set 1 is dipping to undulating	-
									* * * * * * * * * * * * * * * * * * * *	rough fracture surfaces and Set 2 is dipping 60 ro 70 dec	close spacing. grees with	3 -
		2.75 - 4.00		100	100	74				planar rough fracture surface medium spacing.	s and	-
			_									4 -
		4.00 - 4.60		100	92	33			* * * * * * * * * * * * * * * * * * * *			-
			60mm				4.50 21/m	394.35		Lithology: Moderately strong fine grained SANDSTONE w	, grey green /ith minor	
			400mm							quartz veins and minor coars infill along fused fractures.	se sandstone	5 -
		4.60 - 5.90		100	100	44			· · · · · · · · ·	Weathering: Slightly weathe loss of strength apparent, m	red with minor	-
			_						$\begin{array}{cccccccccccccccccccccccccccccccccccc$	on veins and minor orange of select fractures.	xidation on	-
		5.00.0.00		100	100	70				Fractures: 2 apparent sets. 5	Set 1 is dipping	6 -
		5.90 - 6.80		100	100	10			· · · · · · · ·	rough fracture surfaces and Set 2 is dipping 60 ro 70 deg	close spacing. grees with	-
			-				6.80	392.05	;	planar rough fracture surface medium spacing.	∍s and /	
										End of Borehole at 6.	300m	
												8 -
												-
												9 -
Groun	dwater:						Hole In	formatio	n:	Chiselling		
Struck	k, m Ro	ose to After, n	nin Sealeo	d No	Com	ment	Hole D 6	epth (m) .80	Hole Dia (m	m) Casing Dia (mm) Depth Top Dep	th Base Duration	Tool
						Suntered	Equipn	nent:	Deltabase	520		
Remar	ks:								Shift Da	ata: Groundwater Shift 25/11/2016 08:00	Hole Depth (m) Re 0.00 Start	marks t of shift
Boreh	ole terr	ninated at 6.8	80m bgl.							Dry 25/11/2016 18:00	0.00 E bor	rehole.

Photographic Record





pg	priority				Priority Te	/ Geotech I: 021 463	Trial Pit No Trial Pit No TP03A	
	geotechnical				гал www.pri	oritygeot	seconical.ie Sheet 1 of 1	
Proje Name	ct Dereenac	crinnig Wir	ndfarm	Proj P16	ject No.		Co-ords:511344.22 - 551904.85 Date Level: 399.99 25/11/2016	
Locati	Location: Drimoleague, Co. Cork.						Dimensions (m): 3.00 Scale	e
Client	t: Jennings	O'Donnov	van & Partners				Depth:	
iker	Sam	ples & In Situ	u Testing	Depth	Level	Logand	Stratum Description	
Str ⁱ	Depth	Туре	Results	(m)	(m)	Legenu		
	0.60 - 1.30 0.60 - 1.30	B D		0.60	399.39	shte ~ shte , with e ~ shte , shte , sht sht × shte , sht e ^ shte , shte , sht e ^ shte , shte , sht bte × shte , sht shte , shte , sht shte , shte , sht e ^ shte , shte , shte × shte , sht e ^ s	(TOPSOIL) Soft, dark brown silty PEAI. Firm, slightly gravelly sandy SILT with medium cobble content. Gravel is fine to coarse, angular to sub-rounded. Sand is fine to coarse. Cobbles are angular to sub- rounded, 63-150mm dia. Sandstone lithology.	
	1.30 - 1.70 1.30 - 1.70	B D		1.30	398.69		Firm to stiff, blue grey slightly sandy gravelly SILT with medium cobble content. Sand is fine to coarse. Gravel is fine to coarse, angular to sub-rounded, Sandstone	
				1 70	308 20	$\overset{\times}{\times}\overset{\times}{\times}\overset{\times}{\times}\overset{\times}{\times}$	lithology. Cobbles are angular to sub-rounded, 63-200mm dia. Sandstone lithology.	
				1.70	390.23		End of Pit at 1.70m	-
							4	
							5	-
Stabilit	y: Good.					Groundy	water: None encountered.	_
Plant: <u>Backfil</u> Remar	12t track mad I: Arisings. ks: Trial pit term	hine. inated at 1.7	⁷ 0m bgl, bedrock.					_

DCP03A in TP03A

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	3	-	0.60	-
1	2	20	0.62	9
2	2	20	0.62	9
3	3	30	0.63	5
4	16	160	0.76	1
5	4	40	0.64	4
6	1	10	0.61	22
7	4	40	0.64	4
8	2	20	0.62	9
9	1	10	0.61	22
10	2	20	0.62	9
11	1	10	0.61	22
12	1	10	0.61	22

30

Unadjusted CBR, %

9	CBR, Kleyn
8	CBR. TRI

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed

						Priority Tel: 021	Geotechnio 4631600	cal Ltd.			Borehole N	lo.
Pg priority geotechnical						Fax: 02 www.pr	1 4638690 ioritygeoteo	chnical.i	9		BHT04	
	,					<u> </u>					Sheet 1 of 1	
Projec	ct Name	e: Dereer	nacrinnig V	Vindfa	rm	Projec	ct no. 77	(Co-ords:	511565E - 551958N	RC	e
Locati	ion:	Drimole	Cork.				I	_evel:	384.06m OD	Scale 1:50		
Client: Jennings O'Donnovan & Partr					ners			Dates:	23/11/2016	Logged E JMS	Зу	
Well	Water Strike (m)	Depth (m)	Type /Fs (min, max)	CC TCR	oring	(%) ROD	Depth (m) / FI (/m)	Leve (mOD	Legend	Stratum Descri	otion	
		50 (18,21/50 for 0mm) (C) 1.60 - 3.15 3.15 - 3.65 3.65 - 5.05 5.05 - 5.60 5.60 - 6.70	20mm 90mm 120mm 90mm	100 100 100	87 73 93 91 100	0 0 0 0 0	1.60 5/m 16/m 5.05 14/m 6.70	382.4(379.0 377.3(Image: control of the solution	Open hole boring. Driller de Den hole boring. Driller de Lithology: Moderately weak SILTSTONE. Weathering: Slightly weather apparent loss of strength, m colouration and minor clay s fracture surfaces. Fractures: 3 sets apparent. 5-15 degrees with planar to rough fracture surfaces and Set 2 is dipping circa 30 deg planar to stepped smooth fr and close spacing. Set 3 is 40 degrees with planar roug surfaces and close spacing. 2.50 - 2.60m: Mostly non intact 3.00 - 3.15m: Mostly non intact Weathering: Slightly weather vugging on veins, orange on colouration along fracture suf minor clay smearing. Green colouration on select fractur Fractures: 3 sets apparent. 5-15 degrees with planar to rough fracture surfaces and Set 2 is dipping circa 30 deg planar to stepped smooth fr and close spacing. Set 3 is 40 degrees with planar roug surfaces and close spacing. End of Borehole at 6	purple grey red with inor oxidation smearing on Set 1 is dipping undulating close spacing. grees with acture surfaces dipping circa h fracture <i>t.</i> <i>t.</i> <i>t.</i> <i>t.</i> <i>t.</i> <i>t.</i> <i>t.</i> <i>t.</i>	
Groun Struck	dwater: k, m Ro	ose to After, m	nin Sealec	i No	Com	ment	Hole In Hole D	f ormati epth (m) .70	on: Hole Dia (r	nm) Casing Dia (mm) Casing Dia	: oth Base Duration	Tool
							Equipn	nent:	Deltabase	2 520		
Remar Boreh	ks: ole terr	minated at 6.7	70m bgl.						Shift D	Groundwater Shift 23/11/2016 08:00 Dry 23/11/2016 Dry 23/11/2016 Dry 24/11/2016 Dry 24/11/2016 Dry 24/11/2016	Hole Depth (m) Re 0.00 Start 3.15 End 3.15 Start 6.70 E bot bot	emarks t of shfit. of shift. t of shift. ind of rehole.









	•				Priority	Geotech	nical Ltd.	Trial Pit No
priority Tel: 021 genetichnical Fax: 021							1600 38690	TP04A
www.priorityg							echnical.ie S	Sheet 1 of 1
Proje	ct Dereenac	rinnia Wi	ndfarm	Pro	ject No.		Co-ords: 511559.33 - 551956.84	Date
Name):	5		P16	177		Level: 384.52 2	25/11/2016
Locati	i on: Drimoleag	gue, Co. (Cork.				Dimensions (m):	1:25
Client	t: Jennings	O'Donno	van & Partners	1	1	1	Depth: ? 1.05	Logged VT
ater trike	Samp	oles & In Sit	tu Testing	Depth	Level	Legend	Stratum Description	
st≷	Depth	Туре	Results	(m)	(m)	°. \//° °. \//. °. \/	(TOPSOIL) Soft dool brown block dightly grouply al	
	0.20 1.00	P		0.22	384.30	۵۱۵ <u>۵ مالد ۵</u> ۱۱۵ <u>۵ مالد مالد</u> ۱۱ <u>۵ مالد مالد</u> ۲۰۰۰ ۲۰۰ ۲۰۰ ۲۰۰	Firm to stiff, slightly sandy slightly gravelly SILT with	
	0.30 - 1.00	D					medium cobble content and low boulder content. San fine to coarse. Cobbles are angular to sub-rounded, 63-200mm dia. Sandstone lithology. Boulders are ang to sub-angular, 200-400mm dia. Sandstone lithology.	nd is
				1.05	383.47	X X X X X	End of Pit at 1.05m	1
								2
Stabilit	y: Good.				1	Groundv	vater: None encountered.	I
Plant: Backfil	12t track mac I: Arisings.	hine.						
Remar	ks: Trial pit term	inated at 1.	05m bgl, bedrock.					

DCP04A in TP04A

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	10	-	0.20	-
1	8	80	0.28	2
2	7	70	0.27	2
3	0	0	0.2	
4	4	40	0.24	4
5	9	90	0.29	1
6	5	50	0.25	3
7	2	20	0.22	9
8	1	10	0.21	22
9	1	10	0.21	22
10	1	10	0.21	22
11	1	10	0.21	22
12	0	0	0.2	

50

Unadjusted CBR, %

2	CBR, Kleyn	22
5	CBR, TRL	

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





					Priority	/ Geotecł	nnical Ltd.	Trial Pit No	
priority geotechnical					TP05A				
					www.pri	oritygeot	echnical.ie	Sheet 1 of 1	
Projec Name:	t Dereenac	crinnig Wir	ndfarm	Pro	ject No.		Co-ords :511798.49 - 552076.53	Date	
Locatic			Cork		111		Dimensions (m):	Scale	
LUCan		Jue, co. c	JUIK.				Donth:	1:25	
Client:	Jennings	O'Donnov	/an & Partners					VT	
/ater trike	Samp	ples & In Situ	u Testing	Depth	Level	Legend	Stratum Description		
≤ w	Depth 0.00 - 0.80	Type B	Results			Mr X- Mr-	Dark brown black, peaty CLAY. (Topsoil)		
	0.00 - 0.80 0.00 - 0.80	BD		0.50	358.22 357.92 357.22		Dark brown black, peaty CLAY. (Topson) Dark brown black, peaty CLAY with low cobble co Cobbles are 63mm to 200mm dia, angular to sub rounded. Firm, grey blue, slightly sandy gravelly SILT with cobbles can efform to 200mm dia, angular to sub- cobbles are 63mm to 200mm dia, angular to sub- cobbles are 63mm to 200mm dia, angular to sub- cobbles are 63mm to 200mm dia, angular to sub- cobbles are 63mm to 200mm dia, angular to sub- cobbles are 63mm to 200mm dia, angular to sub- cobbles are 63mm to 200mm dia, angular to sub- cobbles are 63mm to 200mm dia, angular to sub- cobbles are 63mm to 200mm dia, angular to sub- cobbles are 63mm to 200mm dia, angular to sub- cobbles are 63mm to 200mm dia, angular to sub- rounded, Sandstone lithology. Boulders are 200m 550mm dia, angular to sub-rounded, Sandstone l End of Pit at 1.50m	intent. medium to ngular. im to ithology. 2 - 3 - 4 - 5 -	
Stability	r: Good.					Ground	water: Trickle flow rate.		
Plant: Backfill:	12T track ma Arisings.	chine.					HUCH HICKIE HOW Fale.		
Remark	s: Trial pit term	inated at 1.5	50m bgl due to obstru	uction.					

DCP05A in TP05A

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	4	-	0.50	-
1	2	20	0.52	9
2	3	30	0.53	5
3	3	30	0.53	5
4	3	30	0.53	5
5	2	20	0.52	9
6	2	20	0.52	9
7	0	0	0.5	
8	2	20	0.52	9
9	2	20	0.52	9
10	2	20	0.52	9
11	2	20	0.52	9
12	1	10	0.51	22

20

Unadjusted CBR, %	6	2, 9	BR,	ed	st	lju	ad	na	U	
-------------------	---	------	-----	----	----	-----	----	----	---	--

9	CBR, Kleyn
-	,,,

12 *CBR, TRL*

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed









					Priority	Geotech	nical Ltd.	Trial Pit N	٧o
P9 priority				Te Fax	1600 38690	TP06A	4		
geotecnincar				www.prioritygeotechnical.ie			Sheet 1 of 1		
Projec	t Dereenad	crinnia Wi	ndfarm	Proj	Project No.		Co-ords: 511542.07 - 552109.44	Date	
Name		5		P16	177		Level: 377.39	24/11/201	16
Locati	Location: Drimoleague, Co. Cork.					Dimensions (m):	1:25		
Client	: Jennings	O'Donnov	van & Partners		I	1	Depth: ? 0.80	Logged VT	ł
ater rike	Sam	ples & In Sit	u Testing	Depth	Level	Legend	Stratum Description		
≥r	Depth	Туре	Results	(m)	(m)			AT I	
	0.20 - 0.80 0.20 - 0.80	BD		0.17	377.22		(TOPSOIL) Dark brown black, gravelly clayey PE, Gravel is fine to coarse, angular t sub-angular, Sandstone lithology. Firm, light brown grey, slightly gravelly slightly sar with medium cobble content. Gravel is fine to coarse. C are angular to sub-rounded, 63-200mm dia. Sand lithology. End of Pit at 0.80m	AT. ndy SILT rse, obbles stone	2
									5 —
Stability	v: Moderate					Groundy	vater: None encountered		5 -
Plant: Backfill	12t track mac : Arisings.	chine.							
Remark	s: Trial pit term	hinated at 0.8	80m bgl, bedrock.						

DCP06A in TP06A

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	6	-	0.20	-
1	7	70	0.27	2
2	8	80	0.35	2
3	4	40	0.39	4
4	4	40	0.43	4
5	2	20	0.45	9
6	2	20	0.47	9
7	2	20	0.49	9
8	3	30	0.52	5
9	1	10	0.53	22
10	1	10	0.54	22
11	2	20	0.56	9
12	1	10	0.57	22
13	0	0	0.57	
14	1	10	0.58	22
15	1	10	0.59	22
16	0	0	0.59	
17	1	10	0.6	22
18	1	10	0.61	22
19	2	20	0.63	9
20	0	0	0.63	

35

Unadjusted	CBR, %
------------	--------

5	CBR, Kleyn	22
7	CBR. TRL	

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





Number:

TP06A

Project Project No Engineer

Derreenacrinnig Wind Farm P16177 Jennings O'Donovan & Partners

Photographic Record





Number:

Derreenacrinnig Wind Farm P16177 Jennings O'Donovan & Partners

					Priority	Geotech	inical Ltd.	Trial Pit	No	
pg	priority Tel: 021 46 Fax: 021 46			l: 021 463 x: 021 463	1600 38690	TP07	Ά			
	www.prioritygeo			oritygeot	echnical.ie	Sheet 1	of 1			
Proje	ct Dereenac	rinnig Wi	indfarm	Proj	ject No.		Co-ords: 511760.44 - 552193.89	Date		
Name	•			P16	177		Level: 348.45	24/11/20	016	
Locati	on: Drimoleag	gue, Co.	Cork.				Dimensions (m):	1:25	;	
Client	: Jennings	O'Donno	van & Partners	I	1	1	Depth: [№]	Logge VT	d	
/ater trike	Samp	oles & In Si	tu Testing	Depth	Level	Legend	gend Stratum Description			
≤ò	Depth	Туре	Results	(11)	(11)		(TOPSOIL) Dark brown black, slightly gravelly PE	AT		
	0.30 - 1.10 0.30 - 1.10	B		0.28	348.17		(IOPSOIL) Dark brown black, slightly gravelly PE Gravel is fine to medium, angular to sub-angular Sandstone lithology. Firm to stiff, light brown grey, slightly sandy grave with medium cobble content. Gravel is fine to coarse. Co are angular to sub-rounded, 63-200mm dia. Sand and Siltstone lithology. End of Pit at 1.20m	AI.		
									5 —	
Stabilit Plant: Backfil	y: Good. 12t track mac I: Arisings.	hine.				Groundy	vater: None encountered.			
Remark	(S: Trial pit termi	inated at 1.	20m bgl, bedrock.							
DCP07A in TP07A

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	5	-	0.30	-
1	3	30	0.33	5
2	4	40	0.37	4
3	2	20	0.39	9
4	2	20	0.41	9
5	2	20	0.43	9
6	2	20	0.45	9
7	3	30	0.48	5
8	1	10	0.49	22
9	3	30	0.52	5
10	1	10	0.53	22
11	3	30	0.56	5
12	1	10	0.57	22
13	0	0	0.57	
14	1	10	0.58	22
15	1	10	0.59	22
16	1	10	0.6	22
17	0	0	0.6	
18	1	10	0.61	22
19	1	10	0.62	22
20	1	10	0.63	
21	1	10	0.64	
22	1	10	0.65	
23	1	10	0.66	
24	1	10	0.67	
25	1	10	0.68	

20

Unadjuste	d CBR, %
-----------	----------

9	CBR, Kleyn	22	
13	CBR, TRL		

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed









	4				Priority	/ Geotecł	nnical Ltd.	Trial Pit	No
pg					Te Fa:	l: 021 463 x: 021 46	31600 38690	TP0	1
					www.pri	oritygeot	echnical.ie	Sheet 1	of 1
Projeo Name	c t Dereenac	rinnig Wir	ndfarm	Proj	ject No. 5177		Co-ords:510809.58 - 550969.85 Level: 216.32	Date 24/11/2(• 016
Locati	on: Drimolea	que, Co. (Cork.	I			Dimensions (m):	Scale	Ð
Client	: Jennings	O'Donnov	van & Partners				Depth:	1:25 Logge VT	∍d
rə ə	Samı	ples & In Sitr	u Testing	Depth	Level			<u>. vi</u>	
Vat Stri	Depth	Туре	Results	(m)	(m)	Legend	Stratum Description		
	0.30 - 0.90 0.30 - 0.90	B D	Results	0.30	216.02		(TOPSOIL) Firm, brown slightly sandy slightly gra SILT. Sand is fine to coarse. Gravel is fine to coar angular to sub-rounded. Light brown grey, silty sandy GRAVEL with mediu cobble content and medium boulder content. Gra fine to coarse, angular to sub-angular, Sandstone lithology. Boulders are angular to sub-angular, 63 dia. Sandstone lithology. End of Pit at 0.90m	ivelly 'se, vel is } i-600mm	2
Stabilit Plant: Backfil	y: Moderate. 12t track mac I: Arisings.	chine.				Ground	water: Water strike at 0.80m bgl. Trickle flow rate.		4
Remain	(3. Trial pit term	inated at 0.9	JUM bgi, bearock.						

DCP01 in TP01

Nr. Blows	Penetration,	pen. Rate,	Depth,	
(cumulative)	cm	mm/blow	m bgl	CBR, % (Kleyn)
0	12	-	0.42	-
1	4	40	0.46	4
2	2	20	0.48	9
3	1	10	0.49	22
4	5	50	0.54	3
5	8	80	0.62	2
6	3	30	0.65	5
8	1	5	0.66	53
9	2	20	0.68	9
10	1	10	0.69	22
11	1	10	0.70	22
12	1	10	0.71	22
13	2	20	0.73	9
14	2	20	0.75	9
16	1	5	0.76	53
18	1	5	0.77	53
19	0	0	0.77	-

20

Unadj	usted	CBR,	%
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9	CBR, Kleyn
12	CBR, TRL

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed









	4				Priority	Geotech	nnical Ltd.	Trial Pit	No
pg	priority geotechnical				Tel Fa:	i: 021 463 x: 021 463	31600 38690	TP02	2
	<u> </u>				www.pri	oritygeot	echnical.ie	Sheet 1	of 1
Projeo Name	ct Dereenac	rinnig Wi	ndfarm	Proj P16	ject No. 5177		Co-ords: 510891.98 - 550989.76 Level: 225.10	Date 24/11/20)16
Locati	i on: Drimoleaç	gue, Co. (Cork.				Dimensions (m):	Scale)
Client	t: Jennings	O'Donno	van & Partners				Depth: $?$ 10	Logge VT	d
ke F	Sam	ples & In Sit	tu Testing	Depth	Level				
Vat Stri	Depth	Туре	Results	(m)	(m)	Legena	Stratum Description		
	0.20 - 0.85	В		0.20	224.90		(TOPSOIL) Firm, brown, slightly gravelly slightly SILT. Sand is fine to coarse. Firm. light brown orange, slightly sandy slightly g	sandy	
	0.20 - 0.85						SILT with high cobble content and low boulder co Sand is fine to coarse. Gravel is fine to coarse, al sub-rounded. Cobbles are angular to sub-rounde 63-200mm dia. Sandstone lithology., Boulders ar angular to sub-angular, 200-500mm dia. Sandsto lithology.	ntent. ngular to d, e ne	
	0.90 - 2.10 0.90 - 2.10	BD		0.85	224.25		Grey brown green, very silty very sandy GRAVEL is fine to coarse, angular to sub-rounded. Sand is coarse. Cobbles are angular to sub-rounded, 63- dia. Sandstone lithology. Boulders are angular to angular, 200-5600mm dia. Sandstone lithology.	Gravel ; fine to 200mm sub-	1
				2.10	223.00		End of Pit at 2.10m		3
									4
									5 —
Stabilit	y: Good.	hing		1	1	Groundv	water: None encountered.		
Backfill Remark	I: Arisings. ks: Trial pit term	inated at 2.	10m bgl, bedrock.						

DCP02 in TP02

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	4	-	0.34	-
1	8	80	0.42	2
2	4	40	0.46	4
3	9	90	0.55	1
4	3	30	0.58	5
5	1	10	0.59	22
6	1	10	0.6	22
8	2	10	0.62	22
9	1	10	0.63	22
10	1	10	0.64	22
11	1	10	0.65	22
13	1	5	0.66	53
15	1	5	0.67	53
17	1	5	0.68	53
18	0	0	0.68	-

10

Unadjusted CBR, %				
22	CBR, Kleyn			
26	CBR, TRL			

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





Number:

TP02

Project Project No Engineer





					Priority	y Geotech	nnical Ltd.	Trial Pit No
pgi	priority geotechnical				Te Fa	l: 021 463 x: 021 46	31600 38690	TP03
				L	www.pri	oritygeot		Sheet 1 of 1
Projec Name	Dereenad	rinnig Wir	ndfarm	Pro P16	ject No.		Co-ords: 511062.64 - 551016.87	Date 24/11/2016
Locati	on: Drimolea	que Co (Cork				Dimensions (m): 3.20	Scale
Locatio							Denth:	1:25
Client	: Jennings	O'Donno\	/an & Partners				2.30	VT
Vater strike	Sam	ples & In Situ	u Testing	Depth	Level (m)	Legend	Stratum Description	
> 00	Depth	Туре	Results				(TOPSOIL) Firm, brown, slightly gravelly slightly	y sandy
	0.20 - 0.40	в		0.18	232.83		SILT. Gravel is fine to coarse, angular to sub-ro Sandstone lithology. Sand is fine to coarse.	unded,
	0.20 - 0.40	D					Firm, light brown, slightly gravelly sandy SILT w cobble content. Gravel is fine to coarse, angula	r to sub-
	0.40 - 1.40 0.40 - 1.40	B D		0.40	232.61	× × × × ×	angular, Sandstone lithology. Sand is fine to concern Cobbles are angular to sub-rounded, Sandston	arse. e
							 lithology. Firm, grey brown, slightly gravelly sandy SILT ways and the second sec	/ith
						× × × × ×	is fine to coarse, angular to sub-rounded, Sand	stone
						$\times \times $	sub-rounded, 63-200mm dia. Sandstone litholo Boulders are angular to sub-rounded, 200-500r	gy.
							Sandstone lithology.	
						× × × × ×		
	1 40 - 2 30	в		1 40	231 61	$\times \times $		
	1.40 - 2.30	D				• × • • × • ×	Silty very sandy GRAVEL with medium cobble of	content.
						• × • • × •		
						*×*****		
						a X		2 -
						• × • • • × • • ×		2 -
				0.00	000 74	• × • • × •		
				2.30	230.71		End of Pit at 2.30m	
								-
								3 -
								-
								4 -
								-
								5 -
Stability Plant:	y: Good. 12t track made	chine.				Groundy	water: None encountered.	i
Backfill Remark	: Arisings. S: Trial pit term	inated at 2 ?	30m bal bedrock					
			, 2001 29., 2001 001.					

DCP03 in TP03

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	5	-	0.25	-
1	4	40	0.29	4
2	4	40	0.33	4
3	2	20	0.35	9
6	1	3	0.36	89
7	2	20	0.38	9
8	1	10	0.39	22
9	3	30	0.42	5
10	1	10	0.43	22
11	1	10	0.44	22
12	15	150	0.59	1
13	5	50	0.64	3
14	4	40	0.68	4
15	3	30	0.71	5
16	2	20	0.73	9
17	3	30	0.76	5
18	1	10	0.77	22
19	2	20	0.79	9
20	2	20	0.81	9
21	1	10	0.82	22
22	0	0	0.82	-

30

Unadi	hatsui	CBR	%
Ullau	usieu	CDR	, /0

9	CBR, Kleyn
8	CBR. TRL

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





Number:

TP03

Project Project No Engineer





	4				Priority	/ Geotech	nnical Ltd.	Trial Pit No	
pgi	priority geotechnical				Te Fa:	TP04			
				<u> </u>	www.pri	oritygeot	rechnical.ie	Sheet 1 of 1	
Projec Name	ct Dereenac	rinnig Wi	indfarm	Proj	ject No.		Co-ords: 511093.42 - 551120.27	Date	
l acati				<u> </u>	177		Level. 235.97 24/11/20 Dimensions (m): 3.00 Scale		
LUCau		Jue, Co. (Denth:	1:25	
Client	: Jennings	O'Donno	van & Partners						
Vater Strike	Samp	ples & In Sit	tu Testing	Depth (m)	Level	Stratum Description			
5 M	Depth	Туре	Results				(TOPSOIL) Dark brown black, slightly sandy grav	elly	
							PEAT. Sand is fine to coarse. Gravel is fine to coa angular to sub-rounded. Sandstone lithology.	arse,	
	0.40 - 1.00 0.40 - 1.00	B D		0.40	235.57		Soft, light brown, slightly gravelly sandy SILT. Gra fine to coarse, angular to sub-rounded, Sandston	ivel is	
						$\overset{\times}{\times}\overset{\times}{\times}\overset{\times}{\times}\overset{\times}{\times}\overset{\times}{\times}$	lithology.		
						× × × × ×			
	1.00 1.70			1.00	224 07	$\times \times \times \times \times$		1	
	1.00 - 1.70	D		1.00	204.01	$\times \times $	Firm to stiff, blue grey, slightly sandy gravelly SIL medium cobble content and low boulder content.	Γ with Sand is	
						× × × × × ×	fine to coarse. Gravel is tine to coarse, angular to rounded, Sandstone lithology.	sub-	
						× × × × ×			
						× × × × ×		-	
				1.70	234.27		End of Pit at 1.70m		
							2 -		
								-	
								3 -	
								4 -	
								=	
								5 -	
Stability Plant:	y: Good.					Groundy	water: None encountered.	I	
Backfill Remark	I: Arisings.	incted at 1	70m hal hodrock						
		Mateu at 1.	/UIII byl, bedrook.						

DCP04 in TP04

	pen. Rate,	Depth,	
Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
15	-	0.55	-
24	240	0.79	0
7	70	0.86	2
2	20	0.88	9
6	60	0.94	2
3	30	0.97	5
4	40	1.01	4
0	_	-	-
	Penetration, cm 15 24 7 2 6 3 4 0 0	pen. Rate, mm/blow 15 - 240 70 200 600 300 400 -	pen. Rate, Depth, Penetration, cm mm/blow m bgl 15 - 0.55 240 0.79 0.86 70 0.88 0.94 200 0.88 0.94 300 0.97 0.94 400 1.01 0

50

Unadjusted (CBR,	%
--------------	------	---

3	CBR, Kleyn	
5	CBR, TRL	

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed









pg	pgl _{priority}				Priority Geotechnical Ltd. Tel: 021 4631600 Fax: 021 4638690							
	geotechnical				www.pri	oritygeot	echnical.ie			Sheet 1	of 1	
Proje Name	ct Dereenac	rinnig Wir	ndfarm	Project No. P16177			Co-ords:511197.23 - 551249.71 Level: 243.22			Date 23/11/2 [,]) 016	
Locati	ion: Drimolea	que, Co. (Cork.				Dimension	s (m):	5.00	Scale	e	
Client	t: Jennings	O'Donnov	van & Partners				Depth:	1.20		Logge VT	ed	
ike	Sam	ples & In Siti	u Testing	Depth	Level	Logand		Stratu	Description			
Str	Depth	Туре	Results	(m) (m) Legend Stratum Description					n Description			
	0.40 - 1.30 0.40 - 1.30	B D		0.40	242.82 241.92		(TOPSOIL) F Soft, brown g medium cob Sand is fine sub-rounded to sub-round Boulders are Siltstone lithe	Firm, dark bro grey, slightly a ble content a to coarse. Gr Sandstone led, 63-200m angular to s ology. End o	wn black, clayey PEAT sandy gravelly SILT wit nd medium boulder con avel is fine to coarse, a lithology. Cobbles are a m dia. Siltstone litholog ub-rounded, 200-1500n	th Intent. Ingular to Ingular Iy. nm dia.	1	
											2	
Stabilit Plant:	ty: Poor. 12t track mac	chine.			1	Groundy	water: None enco	ountered.			1	
Backfil Remarl	II: Arisings. ks: Trial pit term	inated at 1.3	30m bgl, bedrock.									

DCP05 in TP05

0.6 m bgl

Nr. Blows	Penetration,	pen. Rate,	Depth,	
(cumulative)	cm	mm/blow	m bgl	CBR, % (Kleyn)
0	20	-	0.6	-
1	14	140	0.74	1
2	3	30	0.77	5
3	3	30	0.8	5
4	1	10	0.81	22
5	2	20	0.83	9
6	3	30	0.86	5
7	12	120	0.98	1
8	2	20	1	9
9	3	30	1.03	5
15	0	-	-	-

30

5	CBR, Kleyn
8	CBR, TRL

moderate
dry
very dry
not assessed

	4				Priority	y Geotech	nical Ltd.	Trial Pit No	0
PO priority Fax: 021 4631600 geotechnical Priority Fax: 021 4638690 Priority geotechnical in					31600 38690	TP06			
				Bro	www.pri	oritygeou		Sheet 1 of	1
Projec Name	t Dereenac	rinnig Wir	ndfarm	Proj P16	ject No. 3177		Co-oras: 511291.55 - 551357.60 Level: 256.13	Date 23/11/2016	6
Locati	Location: Drimoleague, Co. Cork				Dimensions (m): 3.00	Scale			
Client	: Jennings	O'Donno\	van & Partners				Depth:	1:25 Logged	
er (e	Sam	ples & In Sit	u Testing	Depth	Level			V I	
Vate Strik	Depth	Туре	Results	(m)	(m)	Legend	Stratum Description		
Wa	Depth 0.40 - 0.75 0.40 - 0.75 0.75 - 1.20 0.75 - 1.20	Type B B B B	Results	(m) 0.40 0.75 1.20	(m) 255.73 255.38 254.93	Legenu	(TOPSOIL) Dark brown black, clayey PEAT. Soft to firm, slightly gravelly sandy SILT with high of content. Gravel is fine to coarse. Sand is fine to co Cobbles are angular to sub-rounded, 63-200mm d Sandstone lithology. Firm, light blue grey, slightly gravelly slightly sandy with medium cobble content and low boulder conte Gravel is fine to coarse, angular to sub-rounded. Sandstone lithology. Sand is fine to coarse. Cobble angular to sub-rounded, 60-200mm dia. Sandstone lithology. Boulders are angular to sub-rounded, 200-600mm dia. Siltstone and Sandstone lithology End of Pit at 1.20m	cobble parse. lia. / SILT ent. es are e /.	2
									5 —
Stability Plant: Backfill Remark	y: Good. 12t track mac I: Arisings. (s: Trial pit term	hine. inated at 1.2	20m bgl, bedrock.			Groundv	water: None encountered.		

DCP06 in TP06

Nr. Blows	Penetration,	pen. Rate,	Depth,	
(cumulative)	cm	mm/blow	m bgl	CBR, % (Kleyn)
0	13	-	0.53	-
1	6	60	0.59	2
2	2	20	0.61	9
3	2	20	0.63	9
4	4	40	0.67	4
5	4	40	0.71	4
6	1	10	0.72	22
7	1	10	0.73	22
8	1	10	0.74	22
11	2	7	0.76	37
13	5	25	0.81	7
21	1	1	0.82	309
30	1	1	0.83	359
35	0	0	0.83	-

15

Unadjusted CBR, %

16	CBR, Kleyn
17	CBR, TRL

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





Number:

TP06

Project Project No Engineer





nal	prioritu				Priority Te	/ Geotech I: 021 463	hnical Ltd. Trial Pit No 31600 TD07
Pg	geotechnical				Fax www.pri	x: 021 46 oritygeot	38690 IPU/ technical.ie Sheet 1 of 1
Projec	st _{Demon}			Pro	ject No.		Co-ords:511382.47 - 551532.85 Date
Name: Dereenacrinnig Windfarm			P16	177		Level: 286.25 23/11/2016	
Locatio	Location: Drimoleague, Co. Cork.						Dimensions (m): 3.00 Scale 1:25
Client	: Jennings	O'Donnov	/an & Partners				Depth: N Logged 2.10 VT VT
ater rike	Sam	ples & In Situ	J Testing	Depth	Level	Leaend	Stratum Description
St ≪	Depth	Туре	Results	(m)	(m)		
	0.60 - 1.30 0.60 - 1.30	B D		0.60	285.65		Firm, light brown, slightly gravelly sandy SILT with medium cobble content and medium boulder content. Gravel is fine to coarse, angular to sub-rounded. Sandstone lithology. Sand is fine to coarse. Cobbles are angular to sub-rounded, 63-200mm dia. Sandstone lithology. Boulders are angular to sub-angular to sub-angular to sub-nounded, 1 - 200-1000mm dia. Sandstone lithology.
	1.30 - 2.10 1.30 - 2.10	B D		1.30	284.95		Firm to stiff, blue grey, slightly gravelly sandy SILT with medium cobble content and low boulder content. Gravel is fine to coarse, angular to sub-rounded. Sandstone lithology. Sand is fine to coarse. Cobbles are angular to sub-rounded, 63-200mm dia. Sandstone lithology. BOulders are angular to sub-rounded, 200-600mm dia. Sandstone lithology.
				2.10	284.15		End of Pit at 2.10m 3 4
<u>Ctabilit</u>						Groundy	
Plant: Backfill	12t track mac Arisings.	chine.				Groundy	water: Water strike at 0.10m bgl. Steady flow rate.
Remark	.s: Trial pit term	inated at 2.1	0m bgl, bedrock.				

DCP07 in TP07

Nr. Blows	Penetration,	pen. Rate,	Depth,	
(cumulative)	cm	mm/blow	m bgl	CBR, % (Kleyn)
0	28	-	0.88	-
1	9	90	0.97	1
2	3	30	1	5
3	3	30	1.03	5
4	5	50	1.08	3
5	4	40	1.12	4
6	1	10	1.13	22
7	6	60	1.19	2
8	2	20	1.21	9
9	2	20	1.23	9
10	2	20	1.25	9
11	2	20	1.27	9
12	1	10	1.28	22
20	0	0	1.28	-

25

Unadjusted CBR, %

7	CBR, Kleyn	
10	CBR, TRL	

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





Number:

TP07

Project Project No Engineer





Number:

TP07

Project Project No Engineer

Priority Geote priority geotechnical Priority Geotechnical Fax: 021					/ Geotech I: 021 463 x: 021 46	hnical Ltd. Trial Pit No TP08 TP08	
					www.prioritygeot		technical.ie Sheet 1 of 1
Proje Name	ct Dereenac	rinnig Wir	ndfarm	Proj	ject No.		Co-ords:511449.24 - 551596.66 Date 22/11/2016
Name:				- 10	1//		Level: 293.53 23/11/2010
Locati	on: Drimolea	jue, Co. c	Jork.				Dimensions (m):
Client	t: Jennings	O'Donnov	van & Partners				Depth: ← Loggea 1.60 VT VT
ater rike	Sam	ples & In Siti	u Testing	Depth	Level		Stratum Description
Str Str	Depth	Туре	Results	(m)	(m)	Legena	
	0.50 - 0.75 0.50 - 0.75 0.80 - 1.60 0.80 - 1.60			0.50 0.75 1.60	293.03 292.78 291.93		(TOPSOIL) Firm to stiff, dark brown black, slightly gravelly clayey PEAT with low cobble content. Gravel is fine to coarse, angular to sub-rounded. Cobbles are angular to sub-rounded, 63-200mm dia. Sandstone lithology. Firm to stiff, light brown green, slightly gravelly slightly sandy SILT. Gravel is fine to coarse, angular to sub- rounded. Sand is fine to coarse. Firm, blue green, slightly sandy gravelly SILT with medium cobble content and low boulder content. Gravel is fine to coarse, angular to sub-rounded. Sand is fine to coarse. Cobbles are angular to sub-rounded. Sal S200mm dia. Sandstone lithology. Boulders are angular to sub- rounded. 200-500mm dia. Sandstone lithology. End of PII at 1.80m 2 - .
							5 -
Stabilit Plant: <u>Backfil</u> Remarl	 y: Good to modential for the second second	ərate. :hine. iinated at 1.6	60m bgl, bedrock.			Groundy	lwater: None encountered.

DCP08 in TP08

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	9	-	0.59	-
1	5	50	0.64	3
2	2	20	0.66	9
3	2	20	0.68	9
4	4	40	0.72	4
5	3	30	0.75	5
6	4	40	0.79	4
7	2	20	0.81	9
8	4	40	0.85	4
9	2	20	0.87	9
10	1	10	0.88	22
11	1	10	0.89	22
12	1	10	0.9	22
13	1	10	0.91	22
14	1	10	0.92	22
18	1	3	0.93	
20	0	0	0.93	-

30

Unadjusted CBR	, %
----------------	-----

5	CBR, Kleyn
8	CBR, TRL

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





Priority reotechnical Fax					Priority Te Fa	/ Geotech I: 021 463 x: 021 46	Trial P 31600 TPI 38690 TPI	Trial Pit No TP09	
geotechnical				www.pri	oritygeot	sechnical.ie Sheet	1 of 1		
Project Dereenacrinnig Windfarm			Pro P16	ject No. 3177		Co-ords:511594.68 - 551757.68 Dat Level: 312.98 23/11/2	2016		
Locati	on: Drimolea	gue, Co. (Cork.	I			Dimensions (m): 3.20 Sca	le	
Client	: Jennings	O'Donno'	van & Partners				$- \begin{array}{c c} & & & & \\ \hline \\ Depth: & & \\ \hline \\ 1 40 \end{array} \end{array} \qquad \begin{array}{c c} & & \\ \hline \\ \hline$	1:25	
μo	Sam	 ples & In Sit	tu Testing	Denth			1.40 v i	<u> </u>	
Wate Strik	Depth	Туре	Results	(m)	(m)	Legend	Stratum Description		
	0.00 - 0.30	В					(TOPSOIL) Dark brown, clayey PEAT.		
	0.00 - 0.50							-	
				0.28	312.70	- sile - sile	Firm, light brown green, slightly gravelly CLAY with	-	
	0.40 - 1.40	В		0.40	312.58	$\times \times \times \times \times$	organic content. Gravel is fine to coarse, angular to sub- rounded.		
							Firm, light blue grey, slightly gravelly slightly sandy SILT with medium cobble content and low boulder content.		
						~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	Gravel is fine to coarse, angular to sub-rounded. Sand is fine to coarse. Cobbles are angular to sub-rounded,		
						$\times \times $	63-200mm dia. Boulders are angular to sub-rounded, 200-400mm dia. Sandstone lithology.		
						× × × × × × × × ×			
						$\times \times \times \times$	- - 	-	
						$\overset{(\times,\times,\times,\times)}{\times,\times,\times,\times}$		-	
				1.40	311.58	$\times \times \times \times$	FJ5 Dik-sk # 40mg		
					-		End of Mit at 1.40m	-	
								-	
								2 -	
								-	
								-	
								-	
								-	
								-	
								3 -	
								-	
								4 -	
								5 —	
Stabilit	y: Good.					Groundy	water: Water strike at 1.40m. Trickle flow rate.		
Backfil	I: Arisings.	inine.							
Remar	(s: Trial pit term	inated at 1.4	40m bgl, bedrock.						

#### DCP09 in TP09

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	5	-	0.33	-
1	5	50	0.38	3
2	3	30	0.41	5
3	3	30	0.44	5
4	1	10	0.45	22
5	1	10	0.46	22
6	1	10	0.47	22
7	1	10	0.48	22
8	1	10	0.49	22
9	1	10	0.5	22
10	2	20	0.52	9
12	2	10	0.54	22
14	1	5	0.55	53
15	2	20	0.57	9
16	2	20	0.59	9
17	1	10	0.6	22
18	1	10	0.61	22
19	1	10	0.62	22
20	0		0.62	-

10

Unadjusted CBR, %				
22	CBR, Kleyn			
26	CBR, TRL			

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





Number:

**TP09** 

Project Project No Engineer




					Priority	/ Geotech	nnical Ltd.	Trial Pit N	ю
Pg priority Tel: 021 geotechnical Fax: 021					l: 021 463 x: 021 46	31600 38690	TP10		
	www.priorityge				oritygeot	echnical.ie	Sheet 1 of 1		
Projec	t Dereenac	rinnig Wi	indfarm	Proj	ject No.		<b>Co-ords:</b> 511725.65 - 551895.24	Date	
Name				P16	177		Level: 329.09	23/11/201	6
Locatie	ວ <b>n:</b> Drimoleaດຸ	jue, Co. (	Cork.				Dimensions (m):	5cale 1:25	
Client	: Jennings	O'Donno	van & Partners				<b>Depth:</b> ^[™] / ₊ 1.90	Logged VT	
ater	Samp	ples & In Sit	tu Testing	Depth	Level	l eaend	Stratum Description		
st ≪	Depth	Туре	Results	(m)	(m)	0.0.000		· · · · · · · · · · · · · · · · · · ·	
	0.00 - 0.70 0.00 - 0.70	D		0.70	328 30	الله - عالم و عالم الله - عالم و عالم - ع اله - عالم و عالم - اله - عالم و عالم - - عالم - عالم و عالم - - عالم - عالم - م اله - عالم و م - عالم - عالم - - عالم - عالم       	(TOPSOIL) Dark brown black PEAT with low cool content. Cobbles are angular top sub-rounded, 63-150mm dia.	ble	
	0.90 - 1.40 0.90 - 1.40	B D		0.85	328.24		Firm, light brown, slightly gravelly sandy SILT with medium cobble content and low boulder content. is fine to coarse, angular to sub-rounded. Sand is coarse. Cobbles are angular to sub-0angular, 63- dia. Sandstone lithology. Boulders are angular to angular , 200-300mm dia. Sandstone lithology. Firm to stiff, slight blue grey, slightly sandy gravel with medium cobble content and low boulder con Gravel is fine to coarse, angular to sub-rounded. fine to coarse. Cobbles are angular to sub-rounded. 630-200mm dia. Sandstone lithology. Boulders an angular to sub-rounded, 200-520mm dia. Sandst lithology.	h Gravel s fine to -200mm sub- lly SILT .tent. Sand is ed, re one	1
							End of Pit at 1.90m		2
Stability	y: Good.					Ground	water: None encountered.		
Plant: Backfill	12t track mac Arisings.	;hine.							
Remark	s: Trial pit termi	inated at 1.	90m bgl, bedrock.						

#### DCP10 in TP10

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	5	-	0.75	-
1	9	90	0.84	1
2	3	30	0.87	5
3	2	20	0.89	9
4	3	30	0.92	5
7	4	13	0.96	15
8	2	20	0.98	9
9	1	10	0.99	22
10	1	10	1.00	22
11	2	20	1.02	9
12	1	10	1.03	22
14	2	10	1.05	22
16	1	5	1.06	53
18	1	5	1.07	53
19	2	20	1.09	9
20	1	10	1.10	22
21	1	10	1.11	22
22	1	10	1.12	22
23	1	10	1.13	22
24	1	10	1.14	22
25	1	10	1.15	22
30	5	10	1.2	22
35	0	0	1.2	-

10

Unadjusted CBR, %								
22	CBR, Kleyn							
26	CBR, TRL							

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





TP10

Project Project No Engineer





	Priority Geotechnical Ltd.								No
geotechnical Fax: 02						c: 021 463	1600 38690 setuping	TP11	
						oritygeot		Sheet 1 c	of 1
Projeo Name	ct Dereenad	rinnig Wir	ndfarm	Pro	177		<b>Co-ords:</b> 511851.30 - 552006.60	23/11/20	16
Locati	on: Drimolea		Pork				Dimonsions (m): 3.10	Scale	10
Locati		gue, co. c					Denth:	1:25	4
Client	: Jennings	O'Donno\	/an & Partners	1	1	1	1.15	VT	4
Vater Strike	Sam	ples & In Sit	u Testing	Depth (m)	Level (m)	Legend	Stratum Description		
> 00	Depth	Туре	Results	()	()		(TOPSOIL) Dark brown black, clayey PEAT with	low	
							boulder content.		-
									-
	0.40 - 0.80 0.40 - 0.80	B D		0.40	338.23		Soft to firm, light brown, slightly sandy slightly gra SILT with medium boulder content. Sand is fine t	avelly o	-
						( * * × × × * * * * *	coarse. Gravel is fine to coarse, angular to sub-r Cobbles are angular to sub-rounded, 63-200mm	ounded. dia.	-
						(**X.X.X X X X X) ** X X X	Sandstone lithology.		-
	0.90 - 1.15	В		0.85	337.78	$\begin{array}{c} \times \times \times \times \\ \times \times \times \times \end{array}$	Firm, grey blue, slightly gravelly sandy SILT.		-
	0.30 - 1.13								1 —
				1.15	337.48	<u></u>	End of Pit at 1.15m		-
									-
									-
									-
									-
									2 —
									-
									-
									-
									-
									-
									3 —
									-
									-
									-
									-
									-
									-
									4 —
									-
									-
									-
									-
									-
									-
									5 —
Stabilit Plant:	y: Good. 12t track mac	chine.				Groundv	vater: None encountered.		
Backfil Remark	I: Arisings. (S: Trial pit term	inated at 1.1	15m bgl, bedrock.						

#### DCP11 in TP11

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	7	-	0.47	-
1	12	120	0.59	1
2	17	170	0.76	1
3	2	20	0.78	9
4	2	20	0.8	9
5	2	20	0.82	9
6	2	20	0.84	9
7	3	30	0.87	5
8	8	80	0.95	2
9	8	80	1.03	2
10	4	40	1.07	4
11	1	10	1.08	22
12	5	50	1.13	3
13	3	30	1.16	5
14	7	70	1.23	2
15	0	0	-	-

35

Un	adjusted CBR, %
E	CDD Klown

5	Свк, кіеуп
7	CBR, TRL

Adjustments

# 0.71moderate0.5dry0.35very dry0.5not assessed





TP11

Project Project No Engineer





					Priority	/ Geotech	nnical Ltd.	Trial Pit	No
pgi	Performing Tel: 021 46 Fax: 021 46					31600 38690	TP12	2	
					www.pri	oritygeot	echnical.ie	Sheet 1	of 1
Projec Name:	t Dereenac	rinnig Wir	ndfarm	Proj	ject No. 177		<b>Co-ords:</b> 511906.36 - 552114.95 <b>Level:</b> 349.14	<b>Date</b> 23/11/20	)16
Locatio	on: Drimoleaç	jue, Co. C	Cork.	I			Dimensions (m):	Scale	;
Client:	Jennings	O'Donno\	van & Partners				<b>Depth:</b> 7	Logge VT	d
er ke	Samp	oles & In Situ	u Testing	Depth	Level	Τ			
Wat Stril	Depth	Туре	Results	(m)	(m)	Legend	Stratum Description		
Stability	0.00 - 1.00 1.00 - 2.00	в		2.00	347.14	Stre stre stre stre stre stre stre stre stre stre stre stre stre stre stre stre stre stre stre stre	End of Pit at 2.00m End of Pit at 2.00m		1 2 3 
Plant:	12t track mac	hine.				-	Water ounce at et ronn ogn menne normater.		
Backfill: Remarks	<u>Arisings.</u> s: Trial pit termi	inated at 2.0	J0m bgl, bedrock.						

#### DCP12 in TP12

#### 1.3 m bgl

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	100	1000	1.3	0
1	100	1000	2.3	0
			-	-
		1000		
			Unad	justed CBR, %
			0	CBR, Kleyn
			0	CBR, TRL
	Adjustments		0.71 0.5 0.35 0.5	moderate dry very dry not assessed





TP12

Project Project No Engineer





Priority G priority Tel: ( Fax: (						/ Geotech I: 021 463 IX: 021 46	nical Ltd. 31600 38690	Trial Pi	it No <b>13</b>
	geotechnicai				www.pri	oritygeot	echnical.ie	Sheet 2	1 of 1
Proje Name	Project Dereenacrinnig Windfarm P16177					<b>Co-ords:</b> 511670.28 - 552018.67 <b>Level:</b> 375.78	Dat 22/11/2	. <b>e</b> 2016	
Location: Drimoleague, Co, Cork.							Dimensions (m): 3.10	Sca	le -
Client	t: Jennings	O'Donnov	van & Partners				<b>Depth:</b>		5 jed
ke F	Sam	ples & In Sit	tu Testing	Depth	Level	Τ			
Wat Stril	Depth	Туре	Results	(m)	(m)	Legend	Stratum Description		
	0.00 - 0.30 0.00 - 0.30	B D					Firm, dark brown, peaty CLAY.		
	0.35 - 0.75	В		0.32	375.46	$\begin{array}{c} - \times \\ - \times \\ \times \\$	Firm, grey brown, slightly gravelly sandy S	SILT with	-  -
	0.35 - 0.75	D				× × × × ×	medium cobble content. Sand is fine to co fine to coarse, angular to sub-rounded. Co	arse. Gravel is obbles are	-
						$\overset{\circ}{\times}\overset{\times}{\times}\overset{\times}{\times}\overset{\times}{\times}\overset{\times}{\times}$	63mm to 200mm dia, angular to sub-angu lithology.	lar, Sandstone	
				0.75	375.03	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	End of Pit at 0.75m		
									1 _
									-
									2 -
									-
									-
									-
									- -
									3 -
									-
									-
									-
									-
									-
									-
									4 -
									-
									-
									-
									-
									5 -
Stabilit	ty: Good.					Ground	water: None encountered.		
Plant: Backfil	12T track mar II: Arisings.	chine.							
Remar	ks: Trial pit term	inated at 0.7	75m bgl due to obstr	ruction.		_			

#### DCP13 in TP13

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	6	-	0.36	-
1	3	30	0.39	5
2	2	20	0.41	9
3	3	30	0.44	5
4	1	10	0.45	22
5	2	20	0.47	9
6	1	10	0.48	22
7	1	10	0.49	22
8	1	10	0.5	22
9	3	30	0.53	5
10	1	10	0.54	22
11	2	20	0.56	9
12	3	30	0.59	5
13	3	30	0.62	5
14	3	30	0.65	5
15	1	10	0.66	-

20

Unadjusted CBR, %						
9	CBR, Kleyn					
13	CBR, TRL					

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed









priority				Priority Geotechnical Ltd. Tel: 021 4631600 Fax: 021 4638690				
	Jerround.				www.pri	oritygeot	technical.ie Sheet 1 of 1	
Proje	ct Dereenac		∩dfarm	Project No.			Co-ords:511517.09 - 551958.43 Date	
Name	<u>.</u>			P16	177		Level: 385.00 25/11/2016 3.50 Scale	
Locat	ion: Drimoleag	jue, Co. C	Cork.				Dimensions (m):	
Clien	t: Jennings	O'Donno\	/an & Partners				Depth:         C         Logged           1.30         VT         VT	
ater rike	Samp	ples & In Situ	u Testing	Depth	Depth Level Legend		Stratum Description	
s	Depth	Туре	Results	(m)	(m)		Det brave black alightly gravelly pack (LAV Gravel in	
	0.50 - 1.20 0.50 - 1.20	BD		0.50	384.50		Dark brown black, slightly gravelly peaty CLAY. Gravel is fine to coarse, angular to sub-angular (Topsoil).         Firm, light brown grey, slightly sandy slightly gravelly SILT with medium cobble content, low boulder content. Sand is fine to coarse, angular to sub-rounded. Cobbles are 63mm to 200mm dia, angular to sub-rounded. Boulders are 200mm to 600mm dia, angular to sub-rounded.         Image: start of the s	
2(shill)							5	
Stabilit Plant: <u>Backfil</u> Remar	ty: Good. 12T track man II: Arisings. ks: Trial pit term	chine. inated at 1.3	30m bgl due to obst	ruction.		Groundv	water: None encountered.	

#### DCP14 in TP14

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	8	-	0.58	-
1	7	70	0.65	2
2	13	130	0.78	1
3	2	20	0.8	9
4	2	20	0.82	9
5	1	10	0.83	22
8	1	3	0.84	89
9	1	10	0.85	22
10	1	10	0.86	22
11	1	10	0.87	22
12	1	10	0.88	22
13	1	10	0.89	22
14	1	10	0.9	22
15	1	10	0.91	22
16	1	10	0.92	22
17	1	10	0.93	22
18	1	10	0.94	22
19	1	10	0.95	22
20	1	10	0.96	22
25	0	-	0.96	-

10

Unadiusted CBR	<b>R</b> . %
----------------	--------------

22	CBR, Kleyn
26	CBR, TRL

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed









priority					Priority Te Fa:		Trial Pit No <b>TP15</b>		
Projec				Pro	www.pri	oritygeoi	ecnnical.ie Co-ords:511623.30	- 552060.49	Sheet 1 of 1 Date
Name	Dereenac	crinnig Win	ıdfarm	P16177			Level: 371.22	24/11/2016	
Locati	on: Drimolea	gue, Co. C	Cork.				Dimensions (m):		Scale 1:25
Client	: Jennings	O'Donnov	/an & Partners				Depth: 3.20	1.2	Logged VT
ater rike	Sam	ples & In Situ	J Testing	Depth	Level	Legend	S'	tratum Description	
Str Str	Depth	Туре	Results	(m)	(m)	ALCOUNT			
	0.50 - 1.10 0.50 - 1.10 1.10 - 2.10	B D		0.50	370.72	MC 2016 400 100 100 100 100 100 100 100 100 100	Soft, light brown, graveny Soft, light brown, slig medium cobble conte fine to coarse, angula 63mm to 150mm dia, and Shale lithology.	PEAI. htly sandy gravelly SILT with ent. Sand is fine to coarse. C ar to sub-rounded. Cobles , angular to sub-rounded, Sa	n Gravel is are andstone 1 –
	1.10 - 2.10	D		1.10	510.12		Firm, blue grey, sligh cobble content, low b coarse. Gravel is fine Cobbles are 63mm to rounded, Sandstone 200mm dia, angular t Shale lithology.	medium to ounded. 	
	2.10 - 3.10 2.10 - 3.10	B D							3 -
				3.20	368.02			End of Pit at 3.20m	
									4
									5 -
Stability	y: Good.					Groundv	water: None encountered.		
Backfill	121 track ma		··· · · · · · · ·						
Kemar	(S: Trial pit term	inated at 3.2	.0m bgl due to obstri	uction.					

#### DCP15 in TP15

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	8	-	0.58	-
1	7	70	0.65	2
2	7	70	0.72	2
3	2	20	0.74	9
4	3	30	0.77	5
5	4	40	0.81	4
6	3	30	0.84	5
7	7	70	0.91	2
8	5	50	0.96	3
9	2	20	0.98	9
10	2	20	1.00	9
11	4	40	1.04	4
13	2	10	1.06	22
15	2	10	1.08	22
16	4	40	1.12	4
17	3	30	1.15	5
18	4	40	1.19	4
19	3	30	1.22	5
20	1	10	1.23	22
21	1	10	1.24	22
22	1	10	1.25	22
23	1	10	1.26	22
24	1	10	1.27	22
25	1	-	1.28	-

30

Unadjusted CBR, %								
5 CBR, Kleyn								
8	CBR, TRL							

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





TP15

Project Project No Engineer

	4				Priority	y Geotech	nical Ltd.			Trial Pit	t No
pg	Pg priority Tel: 021 4 geotechnical Fax: 021						31600 38690			TP1	6
					www.pri	oritygeot	echnical.ie			Sheet 1	of 1
Projeo Name	Project Dereenacrinnig Windfarm			<b>Pro</b> j P16	<b>ject No.</b> 177		Co-ords:511441.37 - 551948.82 Level: 396.17			Date 25/11/2	<b>;</b> 016
Locati	on: Drimolea	gue, Co. (	Cork.	ı			Dimensions (m): 4.00			Scale	e :
Client	: Jennings	O'Donnov	van & Partners				<b>Depth:</b> 0.65	1.20		Logge	əd
ike	کے بے Samples & In Situ Testing			Depth	Level	Legend		Stratum Descrir	tion		
Wa Stri	Depth	Туре	Results	(m)	(m)	Legenu		stratum Descrip	otion		
	0.30 - 0.65 0.30 - 0.65	BD		0.28 0.65	395.89		Soft, dark brown bla Gravel is fine to coar medium cobble cont is fine to coarse. Gra rounded. Cobbles ar to sub-rounded. Bou angular to sub-angul	ck, slightly gran rse and sub-an ghtly sandy slig ent, medium b avel is fine to c e 63mm to 200 ilders are 200n lar, Sandstone End of Pit at 0.65	velly peaty CL gular (Topsoil) phtly gravelly S oulder content oarse, angular nm to 400mm 4 and Shale lith im	AY. ). SILT with . Sand to sub- angular dia, ology.	2
											5 —
Stabilit Plant: Backfill	<ul> <li>y: Moderate to g 12T track ma</li> <li>I: Arisings.</li> </ul>	jood. chine.				Groundv	vater: None encountered	1.			
Remark	<b>(s:</b> Trial pit term	inated at 0.6	35m bgl due to obstr	ruction.							

#### DCP16 in TP16

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	8	-	0.38	-
1	3	30	0.41	5
2	1	10	0.42	22
3	2	20	0.44	9
4	1	10	0.45	22
6	1	5	0.46	53
7	1	10	0.47	22
9	1	5	0.48	53
10	1	10	0.49	22
12	1	5	0.5	53
13	2	20	0.52	9
14	2	20	0.54	9
16	1	5	0.55	53
25	0	-	-	-

10

Unadjusted (	CBR, %
--------------	--------

22	CBR, Kleyn	
26	CBR, TRL	

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





Number:

TP16

Project Project No Engineer

	Priority Geotechnical Ltd. Trial Pit					Trial Pit No		
pgi	Depriority geotechnical Fax: 021 4631600 TP17					TP17		
	<u> </u>				www.pri	oritygeot	echnical.ie	Sheet 1 of 1
Projec Name	ct Dereenac	rinnig Wi	indfarm	Proj	ject No.		<b>Co-ords:</b> 511360.97 - 551923.09	Date
l vanie			•l.	<u> </u>	1//		Level: 399.96	<b>Scale</b>
Locau	on: Drimoleau	jue, Co. (	Согк.					1:25
Client	: Jennings	O'Donno	van & Partners				Deptn:	Loggea VT
ater trike	Samp	oles & In Sit	tu Testing	Depth	Level	Legend	Stratum Description	
≥ŵ	Depth	Туре	Results	(m)	(m)	- JAC 316 31	Soft dark brown black DEAT	
	0.25 - 0.55 0.25 - 0.55	BD		0.25	399.73 399.43		Firm, light brown grey, slightly sandy slightly grave SILT. Sand is fine to coarse. Gravel is fine to coar angular to sub-rounded. Cobbles are 63mm to 20 dia, angular to sub-rounded, Sandstone lithology. Boulders are 200mm to 400mm dia, angular to su angular, Shale lithology. End of Pit at 0.55m	אוץ se, 0mm וb
								2 -
								3 -
								4 -
Stabilit	Cood					Ground		5 -
Plant: Backfill Remarl	12T track mar 12T track mar 1: Arisings.	chine.	55m bal due to obstru			Groundy	water: None encountered.	

#### DCP17 in TP17

## 0.33 m bgl

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	8	-	0.33	-
1	4	40	0.37	4
2	2	20	0.39	9
3	3	30	0.42	5
6	2	7	0.44	37
7	1	10	0.45	22
9	1	5	0.46	53
10	1	10	0.47	22
11	1	10	0.48	22
13	1	5	0.49	53
15	0	-	-	-

10

Unadjusted CBR, %				
22	CBR, Kleyn			
26	CBR, TRL			

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





**TP17** 

Project Project No Engineer





na					Priority Te	/ Geotech d: 021 46:	hnical Ltd. Tria	Pit No
Pg	geotechnical				Fax www.pri	x: 021 463 ioritygeot	i38690 Itechnical.ie Shee	<b>ΥΊδ</b>
Proje Name	ct Dereenac	crinnig Wir	ndfarm	Pro	ject No.		Co-ords:511286.32 - 551894.88	<b>Date</b>
Locat	ion: Drimolea			- P10	1//		Level: 398.18 20/	1/2016 cale
Clian							Depth:	
Clien	t: Jennings				1		1.60	VT
Water Strike	Depth		Results	_ Depth (m)	Level (m)	Legend	Stratum Description	
	Depth 0.50 - 0.80 0.80 - 1.40 0.80 - 1.40	Type B D B D	Results	(m) 0.50 0.80 1.40 1.60	(m) 397.68 397.38 396.78 396.58		Dark brown black, peaty CLAY (Topsoil).         Firm, light brown, slightly sandy gravelly SILT with medium cobble content. Sand is fine to coarse, Gravel is fine to coarse, angular to sub-rounded. Cobbles are 63mm to 200mm dia, angular to sub-rounded. Cobbles are 63mm to 200mm dia, angular to sub-rounded. Cobbles are 63mm to 200mm dia, angular to sub-rounded. Cobbles are 63mm to 200mm dia, angular to sub-rounded. Cobbles are 63mm to 200mm dia, angular to sub-rounded. Cobbles are 63mm to 200mm dia, angular to sub-rounded. Sandstone lithology.         Weathered Bedrock.	
Stabilit Plant:	ty: Good. 12T track ma	chine				Groundv	water: None encountered.	
Backfil Remar	I: Arisings. ks: Trial pit term	inated at 1.6	30m bgl due to obsti	ruction.		<u> </u>		

#### DCP18 in TP18

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	5	-	0.55	-
1	3	30	0.58	5
2	2	20	0.6	9
3	1	10	0.61	22
4	2	20	0.63	9
5	1	10	0.64	22
6	2	20	0.66	9
7	0	0	0.66	
8	0	0	0.66	
9	1	3	0.67	89
10	1	10	0.68	22
11	1	10	0.69	22
12	1	10	0.7	22
13	1	10	0.71	22
14	1	10	0.72	22
15	3	30	0.75	5
16	1	10	0.76	22
17	3	30	0.79	5
18	1	10	0.8	22
19	2	20	0.82	9
20	4	40	0.86	4
21	2	20	0.88	9
22	1	10	0.89	22
23	2	20	0.91	9
24	1	10	0.92	22
25	1	10	0.93	22
26	1	10	0.94	22
27	1	10	0.95	22
28	1	10	0.96	22
29	0	0	0.96	-
30	0	0	0.96	-
35	0	-	0.96	-

10

Unadjusted CBR, %				
22	CBR, Kleyn			
26	CBR. TRI			

moderate
dry
very dry
not assessed





**TP18** 

Project Project No Engineer





	4				Priority	y Geotech	nnical Ltd.	I Pit No	
<b>Pg</b> priority Tel: 021 4631600 geotechnical Fax: 021 4638690						31600 38690 T	P19		
governmen www.prior					www.pri	ioritygeot	echnical.ie She	et 1 of 1	
Project Dereenacrinnig Windfarm Projec			vject No. C		Co-ords:511041.42 - 551857.42	<b>Date</b> 12/2016			
Location: Drimoleague, Co. Cork.							Dimensions (m): 3.00	3cale	
Client	t: Jennings	O'Donnov	van & Partners				Depth: 0 Lo	Logged	
ier ke	Sam	Samples & In Situ Testing			Depth Level	Τ		<u> </u>	
Stri	Depth	Туре	Results	(m)	(m)	Legena	Stratum Description		
Stabilit	0.90 - 1.10 0.90 - 1.10	BD		0.90	385.74 385.54	Ground	Soft, dark brown to black, slightly gravelly peaty CLAY. Gravel is fine to coarse, angular to sub-rounded, Sandstone lithology. Soft to firm, light brown grey, slightly sandy gravelly SILT with medium cobble content and low boulder content. Gravel is fine to coarse. Sand is fine to coarse. Cobbles are angular to sub-angular, 63-200mm dia. Sandstone lithology. Boulders are angular to sub-angular, 200-400mm dia. Sandstone lithology. End of Pit at 1.10m		
Plant:	12t track mac	chine.							
Remark	I: Arisings. (S: Trial pit term	 iinated at 1.1	10m bgl, bedrock.						

#### DCP19 in TP19

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	0	-	0.00	-
1	41	410	0.41	0
2	48	70	0.48	2
3	51	30	0.51	5
4	55	40	0.55	4
5	59	40	0.59	4
6	63	40	0.63	4
7	65	20	0.65	
8	65	0	0.65	
9	65	0	0.65	
10	65	0	0.65	
11	65	0	0.65	
12	65	0	0.65	
13	65	0	0.65	
14	66	10	0.66	22
15	66	0	0.66	
16	66	0	0.66	
17	66	0	0.66	
18	66	0	0.66	

40

## Unadjusted CBR, %

4	CBR, Kleyn	22
6	CBR, TRL	

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed




TP19

Project Project No Engineer





pgl	Priority Geot Tel: 021 Fax: 021 Fax: 021 www.priorityc				/ Geotech l: 021 463 x: 021 463 ioritygeot	nnical Ltd. 81600 38690 echnical.ie	Trial Pit No <b>TP20</b> Sheet 1 of 1		
Projeo Name	ct : Dereenac	rinnig Wiı	ndfarm	Pro P16	<b>ject No.</b>		Co-ords:510961.96	6 - 551859.11	Date 22/12/2016
Locati	on: Drimoleaç	gue, Co. (	Cork.	I			Dimensions (m):	3.50	Scale
Client	: Jennings	O'Donno	van & Partners				<b>Depth:</b> 1.40	1.20	Logged VT
Water Strike	Samp Depth	oles & In Site	u Testing Results	_ Depth (m)	Level (m)	Legend	SI	tratum Description	
	0.55 - 1.40 0.55 - 1.40	BD		0.35 0.55	377.37 377.17 376.32		Soft, brown to dark b Firm, slightly sandy, g content. Sand is fine sub-angular to sub-rc angular, 63-200mm c Soft to firm, grey brow high cobble content a fine to coarse. Grave angular.	rown, slightly gravelly peats gravelly SILT with low cobb to coarse. Gravel is fine to bunded. Cobbles are angula dia. Sandstone lithology. wn, slightly sandy gravelly 5 and high boulder content. S d is fine to coarse, angular f	/ CLAY.
									5 -
Stabilit Plant: Backfill Remark	y: Good. 12t track mac I: Arisings. (\$: Trial pit term	hine. inated at 1.4	40m bgl, bedrock.			Groundv	water: Waterstrike at 1.20	m bgl. Trickle flow rate.	

### DCP20 in TP20

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	0	-	0.00	-
1	36	360	0.36	0
2	40	40	0.4	4
3	42	20	0.42	9
4	44	20	0.44	9
5	45	10	0.45	22
6	46	10	0.46	22
7	47	10	0.47	22
8	49	20	0.49	9
9	50	10	0.5	22
10	50	0	0.5	
11	51	10	0.51	22
12	52	10	0.52	22
13	52	0	0.52	
14	53	10	0.53	22
15	55	20	0.55	9
16	57	20	0.57	9
17	59	20	0.59	9
18	60	10	0.6	22
19	61	10	0.61	22

20

## Unadjusted CBR, %

 16
 CBR, Kleyn

 12
 CBR, TRL

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





TP20

Project Project No Engineer





Priority Geote priority geotechnical Fax: 021 www.priority.geotechnical					/ Geotech I: 021 463 x: 021 46	hnical Ltd. Trial Pit N 31600 TP21	No	
Projec				Pro	vww.pri	Oritygeor	technical.ie         Sheet 1 c           Co-ords:510813.20 - 551832.17         Date	of 1
Name:	• Dereenad	rinnig Wir	ndfarm	P16	5177		Level: 349.93 22/12/20	16
Locatio	<b>on:</b> Drimolea	gue, Co. C	Cork.				Dimensions (m): 4.00 Scale 1:25	
Client:	Jennings	O'Donnov	van & Partners				Depth:         N         Logged           2.40         ∨T         ∨T	k
ater rike	Sam	ples & In Situ	u Testing	Depth	Level	Leaend	Stratum Description	
Bt.	Depth	Туре	Results	(m)	(m)	- M&	Coff. dark brown to black slightly candy posty CLAY	
				0.15	349.78	$\frac{1}{2} \times \frac{1}{2} \times \frac{1}$	Solit, datk blown to block, slightly sandy peary class. Sand is fine to coarse. Soft light frown slightly gravely sandy SILT, Gravel is	
							fine to coarse, angular to sub-angular. Sand is fine to coarse. Cobbles are angular to sub-rounded, 63-200mm	
	0.50 - 1.50	в		0.45	349.48	× × × × ×	dia. Soft to firm, grey, slightly gravelly sandy SILT. Gravel is	-
	0.50 - 1.50	D					fine to coarse, angular to sub-angular. Sand is fine to coarse. Cobbles are angular to sub-rounded, 63-200mm	-
							dia. Boulders are angular to sub-rounded, 200-800mm dia. Sandstone lithology.	-
						× × × × ×		1
								1
						× × × × × × × × ×		
						$\times$ $\times$ $\times$ $\times$ $\times$		]
	1.50 - 2.40 1.50 - 2.40	B D						-
						× × × × ×		-
						× × × × ×		-
						× × × × × × × × × ×		2 —
								-
								-
				2.40	347.53	5.000 - 1.0 <b>0</b> - 1.0	End of Pit at 2.40m	-
								-
								-
								- - 2 —
								-
								-
								-
								-
								4 _
								-
								-
								-
								-
								-
								5 —
Stability	r: Good.					Groundy	water: Water encountered at ground level	5
Plant: Backfill:	12t track mac Arisings.	chine.						
Remark	s: Trial pit term	ninated at 2.4	40m bgl, possible bo	oulders or be	edrock.			

#### DCP21 in TP21

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	0	-	0.00	-
1	29	290	0.29	0
2	31	20	0.31	9
3	33	20	0.33	9
4	37	40	0.37	4
5	39	20	0.39	9
6	40	10	0.4	22
7	42	20	0.42	9
8	43	10	0.43	22
9	45	20	0.45	9
10	47	20	0.47	9
11	50	30	0.5	5
12	52	20	0.52	9
13	56	40	0.56	4
14	59	30	0.59	5
15	62	30	0.62	5
16	63	10	0.63	22
17	64	10	0.64	22

20

Unadiusted C	BR. %
--------------	-------

9	CBR, Kleyn	
12	CBR, TRL	

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





TP21

Project Project No Engineer





Priority Geotechnical Ltd. Trial P						Trial Pit No		
pgl	priority				Te Fax	l: 021 463 x: 021 463	1600 38690	TP22
					www.pri	oritygeot	echnical.ie	Sheet 1 of 1
Projec	t Dereenad	rinnig Wir	ndfarm	Pro	ect No.		<b>Co-ords:</b> 510701.89 - 551995.91	Date
Name					Level: 311.55	22/12/2016 Scale		
Locatio	on: Drimolea	gue, Co. C	Cork.				Dimensions (m):	1:25
Client	: Jennings	O'Donno\	/an & Partners				<b>Depth:</b>	Logged VT
ater ike	Sam	ples & In Situ	u Testing	Depth	Level	Legend	Stratum Description	
Str	Depth	Туре	Results	(m)	(m)	Legend		
	0.20 - 0.95 0.20 - 0.95	B		0.95	310.60		Soft to firm, grey, slightly gravelly sandy SILT w medium cobble content and medium boulder or Gravel is fine to coarse. Sand is fine to coarse. are angular to sub-rounded, 63-200mm dia. Bo angular to sub-rounded, 200-500mm dia. Sand lithology. End of Pit at 0.95m	ith ontent. Cobbles ulders are stone 1 - 2 - 3 - 4 -
Stabilit	u: Good					Ground	voter: None anota i	5 -
Plant:	<ul> <li>J2t track mac</li> <li>Arisings</li> </ul>	hine.				Groundy	valei. None encountered.	
Remark	S: Trial pit term	inated at 0.9	95m bgl, bedrock.			1		

#### DCP22 in TP22

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	0	-	0.00	-
1	5	50	0.05	3
2	8	30	0.08	5
3	10	20	0.1	9
4	12	20	0.12	9
5	13	10	0.13	22
6	17	40	0.17	4
7	17	0	0.17	
8	18	10	0.18	22
9	18	0	0.18	
10	18	0	0.18	
11	18	0	0.18	
12	18	0	0.18	
13	18	0	0.18	

20

## Unadjusted CBR, %

9	CBR, Kleyn	22
12	CBR, TRL	

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





TP22

Project Project No Engineer





pgl	Priority Geote Tel: 021 4 Fax: 021 4 www.priorityge					/ Geotech I: 021 463 x: 021 463 ioritygeot	inical Ltd. Trial Pit N Trial Pit N TPS1 TPS1 Stechnical.ie	0
Projec	ct Dereenad	crinnia Wir		Pro	ject No.		Co-ords:511146.34 - 551192.30         Date	<u> </u>
Name	Name: Location: Drimoleague, Co. Cork.			P16	177		Level: 239.83 22/12/201	ô
Locatio							Dimensions (m): 1:25	
Client	Client: Jennings O'Donnovan & Partners					2.10 VT		
Nater Strike	Sam Denth	ples & In Situ	J Testing	_ Depth (m)	Level (m)	Legend	Stratum Description	
2	Debm	Туре				ان مان مان مان مان	کار المعالي Soft, dark brown to black, gravelly very silty PEAT.	
	0.55 - 1.50 0.55 - 1.50 1.50 - 2.10 1.50 - 2.10	B D		2.10	239.48 239.28 237.73	Shice     Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice     Shice       Shice     Shice       Shice	Soft, dark brown to black, gravelly very silty PEAT.         Firm, red brown, slightly gravelly slightly sandy SILT with medium cobble content. Gravel is fine to coarse, angular to sub-angular. Sand stone lithology.         Grey, silty sandy GRAVEL with medium cobble content and medium boulder content. Gravel is fine to coarse, angular to sub-angular. Sand is fine to coarse. Cobbles are angular to sub-angular. Sand is fine to coarse. Cobbles are angular to sub-angular. Sand stone lithology.         Berger (1)       Grey, silty sandy GRAVEL with medium cobble content and medium boulder content. Gravel is fine to coarse. Cobbles are angular to sub-angular. Sand stone lithology.         Berger (2)       Berger (2)         End of Pit at 2.10m       Berger (2)	2
Stabilit	y: Good.					Groundy	water: None encountered.	5 —
Plant: Backfill Remark	12t track mad Arisings. S: Trial pit term	hine. 	I0m bgl, possible b	edrock.				

### DCPS1 in TPS1

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	0	-	0.00	-
1	7	70	0.07	2
2	15	80	0.15	2
3	15	0	0.15	
4	16	10	0.16	22
5	17	10	0.17	22
6	21	40	0.21	4
7	25	40	0.25	4
8	28	30	0.28	5
9	32	40	0.32	4
10	34	20	0.34	9
11	36	20	0.36	9
12	37	10	0.37	22
13	38	10	0.38	22
14	38	0	0.38	
15	38	0	0.38	
16	38	0	0.38	
17	39	10	0.39	22
18	39	0	0.39	

40

## Unadjusted CBR, %

9	CBR, Kleyn	22
6	CBR, TRL	

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed









pgl	Priority Geote Tel: 021 / geotechnical Fax: 021					/ Geotech I: 021 463 x: 021 46	hnical Ltd. Trial Pit No 31600 33690 TPS2
	<b>J</b>			<u> </u>	www.pri	oritygeot	technical.ie Sheet 1 of 1
Projec Name:	t Dereenac	orinnig Wir	ndfarm	Proj	ject No.		Co-ords:511125.90 - 551212.43 Date 22/12/2016
Locati			Cork		111		Level.         243.24         221.12/2010           Dimensions (m):         5.40         Scale
LUcan	Column Dimoleague, Co. Cork.					Donth:	
Client:	Client: Jennings O'Donnovan & Partners						
/ater trike	Sam	ples & In Situ	u Testing	Depth	Level	Legend	Stratum Description
≤v	Depth	Туре	Results	(11)		ALC: 1 9 0 0 10	Soft dark brown to black slightly gravelly peaty CLAY.
						4	
	0.40 - 1.00 0.40 - 1.00	B D		0.40	242.84		Gravelly clayey COBBLES and BOULDERS. Gravel is fine to coarse, angular to sub-rounded. Cobbles are angular to sub-angular, 63-120mm dia. Sandstone lithology. Boulders are angular to sub-angular, 200-1000mm dia. Sandstone lithology.
	1.00 - 2.00	В		1.00	242.24		Firm, blue grey, slightly sandy gravelly SILT with medium
	1.00 - 2.00	D					cobble content and medium boulder content. Sand is fine to coarse. Gravel is fine to coarse, angular to sub- rounded, Sandstone lithology. Cobbles are angular to sub-rounded, 63-200mm dia. Sandstone lithology. Boulders are angular to sub-rounded, 200-400mm dia. Sandstone lithology.
							3
							5
Stability Plant:	/: Good to mod	erate.				Groundy	water: None encountered.
Backfill: Remark	S: Trial pit term	inated at 2.2	2m bgl, possible be	drock.			

#### DCPS2 in TPS2

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	0	-	0.00	-
1	15	150	0.15	1
2	20	50	0.2	3
3	23	30	0.23	5
4	25	20	0.25	9
5	27	20	0.27	9
6	28	10	0.28	22
7	29	10	0.29	22
8	31	20	0.31	9
9	32	10	0.32	22
10	34	20	0.34	9
11	35	10	0.35	22
12	37	20	0.37	9
13	39	20	0.39	9
14	40	10	0.4	22
15	41	10	0.41	22
16	41	0	0.41	
17	42	10	0.42	22
18	42	0	0.42	
19	43	10	0.43	22

20

## Unadjusted CBR, %

9	CBR, Kleyn	22
13	CBR, TRL	

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





TPS2

Project Project No Engineer





	4				Priority	/ Geotech	nnical Ltd.	Trial Pit N	١o
pyı	priority geotechnical				Ter Fa	l: 021 463 x: 021 46	31600 38690	TPS3	1
	<b>3</b> • - •				www.pri	oritygeot	echnical.ie	Sheet 1 o	f 1
Projec Name	t Dereenac	crinnig Wir	ndfarm	Proj ₽1€	ject No.		Co-ords:511160.14 - 551231.59	Date 22/12/201	16
Locati	on: Drimolea	mue. Co. (	Gork.				Dimensions (m): 5.20	Scale	
Client	lennings		van & Partners				Depth: 07	1:25	1
<u> </u>	Sam	nlee & In Sit		T	Τ.	Τ	1.90	VT	
Wate Strike	 Depth	Туре	Results	_ Depth (m)	Level (m)	Legend	Stratum Description		
Wate Strik	Depth 0.50 - 1.10 0.50 - 1.10 1.10 - 1.90 1.10 - 1.90	Type B D B D	Results	0.50 0.50 1.10	241.94 241.34 240.54	Legend	Stratum Description         Dark brown to black, slightly gravelly peaty CLAY         Soft, light brown, slightly sandy gravelly SILT with         coble content and medium boulder content. Sar         to coarse, Gravel is fine to coarse, angular to sub-angular, 63-20         Firm, grey blue, slightly sandy gravelly SILT with         coble content and medium boulder content. Sar         to coarse. Gravel is fine to coarse, angular to angular. Sandstone lithology.         Firm, grey blue, slightly sandy gravelly SILT with         cobble content and medium boulder content. Sar         fine to coarse. Gravel is fine to coarse, angular o         rounded. Cobbles are angular to sub-rounded,         63-200mm dia. Sandstone lithology. BOulders are         angular to sub-angular, 200-700mm dia. Sandston         lithology.         End of Pit at 1.90m	n high nd is fine 200mm sub- medium and is sub- re me	2 - 3
									4
Stability	y: Moderate.					Ground	water: None encountered.		
Plant: Backfill	12t track mac I: Arisings.	chine.				-			
Remark	s: Trial pit term	inated at 1.9	∂0m bgl on boulder la	ayer.					

### DCPS3 in TPS3

Nr. Blows		pen. Rate,	Depth,	
(cumulative)	Penetration, cm	mm/blow	m bgl	CBR, % (Kleyn)
0	0	-	0.00	-
1	12	120	0.12	1
2	46	340	0.46	0
3	55	90	0.55	1
4	70	150	0.7	1
5	78	80	0.78	2
6	83	50	0.83	3
7	84	10	0.84	22
8	87	30	0.87	5
9	88	10	0.88	22
10	89	10	0.89	22
11	92	30	0.92	5
12	94	20	0.94	9
13	95	10	0.95	22
14	95	0	0.95	
15	95	0	0.95	
16	95	0	0.95	
17	95	0	0.95	
18	95	0	0.95	

90

## Unadjusted CBR, %

5	CBR, Kleyn	22
3	CBR, TRL	

0.71	moderate
0.5	dry
0.35	very dry
0.5	not assessed





TPS3

Project Project No Engineer



Tel: 021 4631600 Fax: 021 4638690 www.prioritygeotechnical.ie         Project Name:       Dereenacrinnig Windfarm       Project No. P16177       Co-ords:       511327.38 - 549769.63         Location:       Drimoleague, Co. Cork.       Level:       145.55         Client:       Jennings O'Donnovan & Partners       Dates:       22/12/2016         Depth (m)       Blows/100mm	DP02 Sheet 1 of 1 Hole Type DP Scale 1:25
www.prioritygeotechnical.ie         Project Name:       Dereenacrinnig Windfarm       Project No. P16177       Co-ords:       511327.38 - 549769.63         Location:       Drimoleague, Co. Cork.       Level:       145.55         Client:       Jennings O'Donnovan & Partners       Dates:       22/12/2016         Depth (m)       10       20       30       40	Sheet 1 of 1 Hole Type DP Scale 1:25
Project Name:     Dereenacrinnig Windfarm     Project No. P16177     Co-ords:     511327.38 - 549769.63       Location:     Drimoleague, Co. Cork.     Level:     145.55       Client:     Jennings O'Donnovan & Partners     Dates:     22/12/2016       Depth (m)     10     20     30     40	Hole Type DP Scale 1:25
Location:     Drimoleague, Co. Cork.     Level:     145.55       Client:     Jennings O'Donnovan & Partners     Dates:     22/12/2016       Depth (m)     Blows/100mm	Scale 1:25
Client:     Jennings O'Donnovan & Partners     Dates:     22/12/2016       Depth (m)     10     20     30     40	
Depth Blows/100mm (m) 10 20 30 40	Logged By
	Torque (Nm)
$\begin{array}{c c} 2 \\ 2 \\ 4 \\ 1 \\ 3 \\ 4 \\ 5 \\ 5 \\ \end{array}$	
13 25	
3	
4	
Remarks:       Fall Height:       500       Cone Base Dia:         DP02 terminated at 1.90m bgl, refusal.       Hammer Wt:       50       Final Denth:       1 00	
Probe Type: DPH	

Tel: 021 4631600 mww.prioritygeotechnical.is         Project No. P16177       Co-ords: 511264.03 - 550031.30         Cocation:       Drimoleague, Co. Cork.       Level:       156.43         Depth       Blows/100mm         (m)       10       20       30       40         4       4       3       5       6       6       5         20       30       40       40       40       40       40         1       10       20       30       40       40         21       23       24       24       24       24       24         3       3       3       3       3       40       40         3       3       3       3       40       40       40         3       3       3       3       5       40       40       40         3       3       3       3       40       40       40       40       40       40       40       40       40       40       40       40       40       40       40       40       40       40       40       40       40       40       40       40       40       40       40 <th>DP03 Sheet 1 of 1 Hole Type DP Scale 1:25 Logged By</th>	DP03 Sheet 1 of 1 Hole Type DP Scale 1:25 Logged By
www.prioritygeotechnical.ie           Project No. P16177         Co-ords:         511264.03 - 550031.30           Location:         Drimoleague, Co. Cork.         Level:         156.43           Client:         Jennings O'Donnovan & Partners         Dates:         22/12/2016           Depth         Blows/100mm           (m)         10         20         30         40           1         5         6         5         6         5           4         4         3         5         6         5           2         2         20         20         20         40           3         5         6         5         21         25           3         3         5         21         25         21           3         3         5         21         25         21	Sheet 1 of 1 Hole Type DP Scale 1:25 Logged By
Project Name:     Dereenacrinnig Windfarm     Project No. P16177     Co-ords:     511284.03 - 550031.30       Location:     Drimoleague, Co. Cork.     Level:     156.43       Client:     Jennings O'Donnovan & Partners     Dates:     22/12/2016       Depth (m)     10     20     30     40       1     3     5     6     5       2     3     3     4     4       3     3     21     25	Hole Type DP Scale 1:25 Logged By
Joint         Level:         156.43           Cilent:         Jennings O'Donnovan & Partners         Dates:         22/12/2016           Depth (m)         Blows/100mm           1         3         5         6         5           2         3         3         3         4         4           3         3         3         3         4         4           3         3         3         3         4         4         4           3         3         3         3         4         4         4         4           3         3         3         3         4         4         4         5         5         4         4         4         5         5         21         25         4         4         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5         5<	Scale 1:25 Logged By
Client:     Jennings O'Donnovan & Partners     Dates:     22/12/2016       Depth (m)     10     20     30     40       1     3     5     1     1       3     3     21     25	Logged By
Depth (m) 10 20 30 40 4 4 4 3 3 21 25 21 25 3 3 3 3 3 40 4 4 4 5 5 4 4 5 5 4 4 5 5 4 4 5 5 4 4 5 5 4 4 5 5 4 4 5 5 4 5 5 5 7 1 5 5 7 7 7 7 7 7 7 7 7 7 7 7 7	
(m)     10     20     30     40       3     5     6     6     6       4     4     3     3     1       5     5     21     25	Torque
	(Nm)
3	
Pomarke: Eall Height: 500 Cone Page Die:	
DP03 terminated at 1.40m bgl, refusal. Hammer Wt: 50 Final Depth: 1.40	
Probe Type: DPH	

			Priority	Geotechnica	al Ltd.			Pr	obe No
	priority         Tel: 021 4631600           geotechnical         Fax: 021 4638690						C	DP04	
geotec	www.prioritygeotechnical.ie						She	et 1 of 1	
Project Name:	Dereenacrinnig Wi	ndfarm	<b>Project No.</b> P16177	Co	-ords:	511076.27 - 55	0454.73	Ho	DP
Location:	Drimoleague, Co.	Cork.		Lev	/el:	174.95			Scale 1:25
Client:	Jennings O'Donnov	van & Partners		Dat	tes:	22/12/2016		Lo	gged By
Depth (m)	10	)	Blow	s/100mm	30	4	0		Torque (Nm)
		·							
-	6	10							
-	3								
-	2								
-	0								
-	1								
E ─ 1	2								
-		11	20						
				25					
-									
-									
-									
-									
- 2									
- -									
- -									
-									
-									
- 3									
-									
-									
-									
-									
- 4 -									
-									
-									
Remarks:	1		Fall Height:	500		Cone Base Dia	a:		
DP04 terminated at 1.20m bg, refusal. Hammer Wt: 50 Final Depth: 1.20									
			Probe Type:	DPH					

Tel: 021 4631600 Fax: 021 4638690 www.prioritygeotechnical.ie         Project Name:       Dereenacrinnig Windfarm       Project No. P16177       Co-ords:       511028.37 - 550647.52         Location:       Drimoleague, Co. Cork.       Level:       186.97         Client:       Jennings O'Donnovan & Partners       Dates:       22/12/2016         Depth (m)       Blows/100mm	DP05
www.prioritygeotechnical.ie       Project Name:     Dereenacrinnig Windfarm     Project No. P16177     Co-ords:     511028.37 - 550647.52       Location:     Drimoleague, Co. Cork.     Level:     186.97       Client:     Jennings O'Donnovan & Partners     Dates:     22/12/2016       Depth (m)     10     20     30     40	
Project Name:       Dereenacrinnig Windfarm       Project No. P16177       Co-ords:       511028.37 - 550647.52         Location:       Drimoleague, Co. Cork.       Level:       186.97         Client:       Jennings O'Donnovan & Partners       Dates:       22/12/2016         Depth (m)       10       20       30       40	Sheet 1 of 1
Location:     Drimoleague, Co. Cork.     Level:     186.97       Client:     Jennings O'Donnovan & Partners     Dates:     22/12/2016       Depth (m)     Blows/100mm	Hole Type
Client:     Jennings O'Donnovan & Partners     Dates:     22/12/2016       Depth (m)     10     20     30     40	<b>Scale</b> 1:25
Depth (m)         Blows/100mm           10         20         30         40	Logged By
	Torque (Nm)
$ \begin{array}{c} 0\\ 2\\ 4\\ 4\\ 4\\ 4\\ 4\\ 6\\ 13\\ 15\\ 18\\ 19\\ 19\\ 19\\ 19\\ 19\\ 19\\ 19\\ 19\\ 19\\ 19$	
3	
Remarks:       Fall Height:       500       Cone Base Dia:         DP05 terminated at 1.50m bg, refusal.       Hammer Wt:       50       Final Depth:       1.20	
Probe Type: DPH	——

			Priority Geotecl	nnical Ltd.			Pr	obe No
	Drity Innical		Tel: 021 4631600 Fax: 021 4638690					DP06
geotet	www.prioritygeotechnical.ie					She	et 1 of 1	
Project Name:	Dereenacrinnig Windfarm	<b>Projec</b> P1617	ct No. 7	Co-ords:	510967.71 - 55	0739.18	Ho	DP
Location:	Drimoleague, Co. Cork.			Level:	191.63			Scale 1:25
Client:	Jennings O'Donnovan & Partners			Dates:	22/12/2016		Lo	gged By
Depth (m)	10	20	Blows/100n	nm 30	4	0		Torque (Nm)
- 1	2 5 10 10 3 3 3 8 9	16	19					
- 2								
3								
- 4								
-								
Remarks [.]		Fall F	leight: 500	1	Cone Base Dia	a:		
DP06 termi	nated at 1.10m bgl.	Hamr	mer Wt: 50		Final Depth:	1.10		
		Prob	e Type: DPH		• • •			

		Priority Geotec	hnical Ltd.			Probe No	
	priority         Tel: 021 4631600           geotechnical         Fax: 021 4638690						
geotec	www.prioritygeotechnical.ie						
Project Name:	Dereenacrinnig Windfarm	<b>Project No.</b> P16177	Co-ords:	510868.00 - 550	0802.05	Hole Type DP	
Location:	Drimoleague, Co. Cork.	-	Level:	198.34		<b>Scale</b> 1:25	
Client:	Jennings O'Donnovan & Partners	3	Dates:	22/12/2016		Logged By	
Depth (m)	10	Blows/100r	nm		•	Torque (Nm)	e
-		20	30	4	0	()	
-	0						
	3						
	2						
-							
-	1						
- - - 1	3						
- '	0 4						
-	2						
	6						
 - -	8						
	9						
-	10	22					
- 2		25					
-							
-							
-							
-							
- 3							
-							
-							
-							
-							
– 4 -							
-							
E							
-							
E E							
-							
Remarks:		Fall Height: 500		Cone Base Dia	a:	I	
DP07 termi	inated at 2.00m bgl, refusal.	Hammer Wt: 50		Final Depth:	2.00		
		Probe Type: DPH					

		Priority Geotech	nnical Ltd.			Probe No
	<b>prity</b>	Tel: 021 463 Fax: 021 46	DP08			
	www.prioritygeotechnical.ie S					Sheet 1 of 1
Project Name:	Dereenacrinnig Windfarm	Project No. P16177	Co-ords:	510787.80 - 550844	4.01	Hole Type DP
Location:	Drimoleague, Co. Cork.		Level:	202.37		<b>Scale</b> 1:25
Client:	Jennings O'Donnovan & Partners		Dates:	22/12/2016		Logged By
Depth (m)	10	Blows/100m	nm 30	40		Torque (Nm)
1		19 22 25				
- 2						
3						
- 4						
		, 	1			
Remarks:	nated at 0.60m bol. refusal	Fall Height: 500		Cone Base Dia:	0.00	
Hammer Wt: 50   Final Depth:						
		Prope type: DPH				

### **APPENDIX B**

### **GEOPHYSICAL REPORT**

Dereenacrinnig Wind Farm,

Geophysical Investigation

Priority Geotechnical Ltd. (PGL)

P16177_GP_Rp_F01, March, 2017



Unit 12, Owenacurra Business Park, Midleton, Co. Cork

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## **DERREENACRINNIG WIND FARM**

## **GEOPHYSICAL INVESTIGATION**

## REPORT

## No. P16177_GP_Rp_F01

**Client:** 

George O'Mahony & Associates

**Consulting Engineers:** 





## **REPORT CONTROL SHEET**

Client		George O'Mahony & Associates						
Engineer Representative	Itative Jennings O'Donovan Consulting Engineers							
Project Name	oject Name Derreenacrinnig Wind Farm Geophysical Investigation							
Document Name Technical Report								
Project Number		P16177						
This Report	тос	Text	No. of Volume	No. of Appendices	Drawings	Electronic data		
Comprises of	1	32	1	4	2	*.pdf *.dwg		

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# A) EXECUTIVE SUMMARY

Priority Geotechnical Ltd. was instructed by Jennings O'Donovan Consulting Engineers on behalf of George O'Mahony & Associates to undertake a geophysical survey in conjunction with a site investigation survey at Derreenacrinnig Wind Farm.

The geophysical survey comprised of Electrical Resistivity Soundings, Seismic Refraction Profiling and Multi-Channel Analysis of Surface Wave (MASW) at 7 proposed turbine locations as well as a proposed substation location.

A full methodology of the above survey techniques is provided as part of this report.

The survey was carried out between 12th and 14th December 2016.

The survey objectives were as follows:

- Establish s-wave seismic velocity to depth at the 7 no. turbine bases.
- Collect electrical resistivity soundings across the 7 no. turbine bases as well as additional soundings at the substation site.

Direct resistance values were recorded at each of proposed locations at various electrode spacings. Direct resistance values (ohm) were converted to soil resistivities (ohm-m), the results are given in APPENDIX B. A location map showing the surveyed locations is given in APPENDIX A.

All shots collected for MASW analysis during this project were processed using the Seisimager / SW Surface wave analysis software.

Initial phase velocities were attempted to be derived for each shot from each spread. No dispersion curves were apparent for all shots at all 7 no. turbine bases with all wave propagation data within the initial 30ms of each record.

The reason no MASW data was present on the data is that the fundamental node of dispersion is not very sensitive with high velocities present near the surface as is the case on this site.

Fundamentally the MASW system is better setup to assess the stiffness characteristics of overburden material rather than bedrock.

In order to provide the required engineering moduli s-wave velocities have been derived from the calculated p-wave velocities (from the seismic refraction methodology) using a ratio of 1:3 (s-wave:p-wave). This is essentially approximating s-wave velocities so caution should be taken with the engineering moduli extrapolated from this approximation. The Poisson's Ratio presented (0.44 for all profiles) is of no use as this is calculated based on the ration of s-wave to p-wave velocity. These results are presented in APPENDIX C.

# **B)** INTRODUCTION

## B.1) Scope of Works

Priority Geotechnical Ltd. (herein referred to as PGL) was instructed by Jennings O'Donovan Consulting Engineers on behalf of George O'Mahony & Associates to undertake a geophysical survey in conjunction with a site investigation survey at Derreenacrinnig Wind Farm.

The geophysical survey comprised of Electrical Resistivity Soundings, Seismic Refraction Profiling and Multi-Channel Analysis of Surface Wave (MASW) at 7 proposed turbine locations as well as a proposed substation location.

The survey was carried out between 12th and 14th December 2016.



Figure A.1: Satellite image showing turbine and substation location (Google Earth).

## **B.2)** Survey Objectives

The survey objectives were as follows:

- Establish s-wave seismic velocity to depth at the 7 no. turbine bases.
- Collect resistivity soundings across the 7 no. turbine bases as well as additional soundings at the substation site.

## **B.3)** Site Topography

The proposed survey area is in a mountainous area with peat bogs located between rock outcrop. Topography along the survey route ranged from approximately 240m to 380m (OD Malin). All survey work was carried out on foot by a three person field crew.

## B.4) Site Geology

According to the GSI 100k Geology Map (Fig. A.2) the survey area is underlain by the Gun Point Formation, described purple and green medium-coarse grained sandstones (locally pebbly) with thin interbedded purple siltstones.



Figure A.2: GSI 100k Bedrock Geology Map of the site. The Gun Point Formation is shown in transparent red underlying the survey area at Derreenacrinnig West.

#### B.4.i) Teagasc Soils Map

According to the Teagasc Soils Map the site is underlain by shallow soils derived from noncalcareous rock or gravels with/without peaty surface horizon.



Figure A.3: Teagasc Soils Map showing the site to be underlain by shallow soils derived from noncalcareous rock or gravels with/ without peaty surface horizon (shown in pink).

# C) SURVEY METHODOLOGY

The geophysical survey comprised of the following methods;

- Electrical Resistivity Sounding
- Seismic Refraction Profiling
- Multi-Channel Analysis of Surface Wave (MASW)

The methodology for each technique used in this project is outlined below.

#### C.1) Electrical Soil Resistivity Sounding

DC resistivity measurements were made by injecting a DC current into the ground through two current electrodes and measuring the resulting voltage at the surface between two potential electrodes. This method measures the bulk electrical resistivity which is a function of soil and rock matrix, percentage of fluid saturation and the conductivity of the pore fluids.

Resistivity measurements were taken at the 7 no. turbine bases as well as additional measurements at the substation site.

#### C.1.i) Data Acquisition

A GEO Fluke 1623 Ground / Earth Tester with accompanying cable and stainless steel electrodes were used to collect the data. Saline water was used to ensure proper contact resistance was maintained between the electrodes and the ground.

The Wenner 4-Point Method was used to determine ground resistivity's. This method involves placing 4 no. electrodes at varying equal spacing to approximate the soil resistivity as it varies with depth. Electrode spacings of 0.5m, 1.0m, 2.0m 5.0m, 10.0m, 20.0m and 40.0m were used for all locations.

The results provided are given as apparent resistivity and refer to a bulk resistivity of the soil with depth.

#### C.1.ii) Data Processing

Direct resistance values were recorded at each of the 7 no. electrode spacings. Direct resistance values (ohm) were converted to soil resistivities (ohm-m) using the following formula;

 $2 * \pi * electrode spacing (m) * direct resistance (ohm)$ 

#### C.2) Multi-Channel Analysis of Surface Wave Profiling

In the MASW survey method a surface wave is generated by a source (sledge hammer) at the surface resulting in the generation of a surface wave. Surface waves, often referred to as 'ground roll' are characterized as being of low velocity, low frequency and relatively high amplitude.

Surface waves are dispersal, with the frequency range of the surface waves all having different velocities. The surface wave phase velocity spectrum (the velocity of the different frequencies of surface waves) is a good proxy for shear wave velocities. The frequency range of the surface wave spectrum determines the depth of investigation possible. Lower frequencies see deeper and higher frequencies see shallower.

A total of 7 no. seismic refraction profiles were acquired centred on the 7 no. turbine bases. Some profiles were moved slightly to avoid steep inclines.

#### C.2.i) Data Acquisition

A 24-channel Geometrics Geode seismic system was utilized with a 24-channel seismic multicore cable, 4.5Hz geophones, and a sledge hammer and plate as a seismic source. A geophone spacing of 1m with a profile length of 23m was used for all profiles.

Data was recorded using SGOS Seismodule Controller software. A total of 16 shots were undertaken on each seismic line;

- 4 shots were taken 1m off the end of each side of the survey profiles
- 4 shots were taken 2m off the end of each side of the survey profiles

No data acquisition filters were used in the acquisition of the MASW shots.

## C.2.ii) Data Processing

All shots collected for MASW analysis during this project were processed using the Seisimager / SW Surface wave analysis software.

Initial phase velocities were attempted to be derived for each shot from each spread. No dispersion curves were apparent for all shots at all 7 no. turbine bases with all wave propagation data within the initial 30ms of each record.

The reason no MASW data was present on the data is that the fundamental node of dispersion is not very sensitive with high velocities present near the surface as is the case on this site.

Fundamentally the MASW system is better setup to assess the stiffness characteristics of overburden material rather than bedrock.

#### C.2.iii) Data Interpretation

No s-wave results were interpreted based on the issues as outlined above.

#### C.3) Seismic Refraction Profiling

Seismic refraction measurements are made by measuring the travel time of direct and refracted acoustic waves as they travel from the surface through one layer to another and back to the surface where their arrival times are recorded. The travel tie is a function of the seismic or acoustic velocity and geometry of the subsurface layers of soil and rock.

A total of 7 no. seismic refraction profiles were acquired centred on the 7 no. turbine bases. Profiles were named S1 through to S7 and are consecutive with the turbine base numbers. Some profiles were moved slightly to avoid steep inclines.

#### C.3.i) Data Acquisition

A 24-channel Geometrics Geode seismic system was utilized with a 24-channel seismic multicore cable, 4.5Hz geophones, and a sledge hammer and plate as a seismic source. A geophone spacing of 1m with a profile length of 23m was used for all profiles.

Data was recorded using SGOS Seismodule Controller software. A total of 7 shots were undertaken on each seismic line; 2 end-shots, 2 off-shots and 3 mid-shots. To improve signal to noise ratio, individual hammer shots were stacked at each shot location where necessary.

#### C.3.ii) Data Processing

Data processing was undertaken utilizing Seisimager Seismic 2D software programs. Surveyed topography was input for each seismic spread. First breaks were picked after which a tomographic inversion was computed using travel-time computation via ray-tracing. Velocity modeling and travel time plots were constructed for each spread. Seismic velocity phases were picked and the thickness of each velocity unit calculated using the intercept-time method.

RMS errors of the tomographic inversion is summarised in Table C.1.

Spread no.	<b>RMS Error</b>
S1	0.71ms
S2	0.78ms
S3	0.54ms
S4	0.25ms
S5	0.11ms
S6	0.61ms
S7	0.78ms

Table C.1: RMS Error of inverted seismic data

#### C.3.iii) Data Interpretation

It should be noted that when layer thicknesses are modelled from the seismic data the areas of greatest coverage (i.e. the centre of the spread) will have the greatest accuracy. At the edges of the spread less ray coverage reduces the accuracy of layer interpretation and thickness calculation.

Approximate errors for velocities are estimated to be +/-10%. Errors for the calculated layer thickness are of the order of +/-15%. Possible errors due to the "hidden layer" and "velocity" effects may also occur (Soske, 1959). Seismic Refraction generally determines the depth to horizontal or near horizontal layers where the compaction/strength/rock quality changes. Where low velocity layers are present or where layers dip with more than 20 degrees angle the accuracy becomes less.

The results of the seismic refraction survey are given in APPENDIX C.

## C.4) GNSS Surveying

Horizontal control and Elevation was provided by a Trimble VRS (Real Time Kinematic/Virtual Reference Station) enabled GPS. Survey Controller software was used to provide high-accuracy, GNSS positioning.

All positions are plotted in Irish Transverse Mercator (ITM). Elevations are to O.D. Malin.

## D) SURVEY RESULTS

#### D.1) Electrical Soil Resistivity Sounding

The results of this method are given in APPENDIX B. Location maps showing the surveyed locations are given in APPENDIX A. For the 7 no. turbine bases the location of the soundings is the same as the centre of the seismic refraction profiles.

## D.2) MASW

As outlined in Section B.2.ii, due to the nature of the site no s-wave data was present on the recorded data. Section B.2 outlines that best practice was followed for the acquisition of MASW profiles.

In order to provide the required engineering moduli s-wave velocities have been derived from the calculated p-wave velocities (from the seismic refraction methodology) using a ratio of 1:3 (s-wave:p-wave). This is essentially approximating s-wave velocities so caution should be taken with the engineering moduli extrapolated from this approximation. The Poisson's Ratio presented (0.44 for all profiles) is of no use as this is calculated based on the ration of s-wave to p-wave velocity. These results are presented in APPENDIX C.

#### D.3) Seismic Refraction

As part of the MASW acquisition additional shots were acquired for p-wave refraction data acquisition. Modelled seismic velocities (Vp) ranged from c. 300 to 3500 m/s for the soil and bedrock materials.

P-Wave velocity (m/s)	Interpretation
0 - 500	Peat
500 -1000	Firm OVERBURDEN / Highly fractured
500-1000	SILTSTONE / SANDSTONE
1000 - 1800	Fractured SILTSTONE / SANDSTONE
1800 - 2600	Slightly fractured SILTSTONE / SANDSTONE
> 2600	Fresh SILTSTONE / SANDSTONE

The seismic refraction data has generally been interpreted on the following basis:

Table B.2.1: Interpretation based on P-Wave velocity

The results of the seismic refraction survey are presented in APPENDIX C as Vp soundings at the turbine bases. Additionally the seismic refraction profiles are presented as cross sections with

p-wave seismic velocity displayed to depth. The orientation for all profiles is west towards east. Location maps showing the surveyed locations are given in APPENDIX A.

# D.4) Estimated Excavatability

The rippability of the bedrock can be estimated by the seismic refraction method. The seismic velocity of the rock concerned is compared with a chart of ripper performance based on ripping operations in a wide variety of rock (see Fig. C.1).



Figure C.1: P-Wave Seismic Velocity in relation to rippability (Bell, 1992)

The excavation of each of the 7 no. turbine bases will encounter sedimentary bedrock at shallow depths or at the surface. In general, it can be said that where P-Wave velocity exceeds 2500m/s rock will be marginal rippable to non-rippable. Where velocities are less than 2500m/s this would indicate probable rippability. However, if the fractures and bedding joints do not allow tooth penetration the material may not be ripped.

# **APPENDIX A: DRAWINGS**

Locations maps have been produces to show seismic refraction and electrical sounding locations and are summarised below. Note that electrical soundings were taken at the same location as the seismic refraction soundings displayed as blue circles on the drawings.

Drawing Number	Geophysical profile	Scale
P16177_Gp_D01	S1-S4 and Substation	1:500 at A1
P16177_Gp_D02	S5-S7	1:500 at A1







# **APPENDIX B: SOIL RESISTIVITY SOUNDINGS**

P16177 - Derreenacrinnig Wind Farm		
Date:	13/12/2016	
T1		
Easting [ITM]		510915
North	551849	
Elevatior	n [OD Malin]	373.1
	Direct	
Electrode	resistivity	Soil resistivity
• ·	(-)	1- 1
Spacing	(Ω)	(Ωm)
Spacing 0.5	<b>(Ω)</b> 99.1	<b>(Ωm)</b> 311.3
Spacing 0.5	(Ω) 99.1 74.0	(Ωm) 311.3 464.9
Spacing           0.5           1           2	(Ω) 99.1 74.0 76.8	(Ωm) 311.3 464.9 965.1
Spacing           0.5           1           2           5	(Ω) 99.1 74.0 76.8 39.4	(Ωm) 311.3 464.9 965.1 1237.8
Spacing           0.5           1           2           5           10	(Ω) 99.1 74.0 76.8 39.4 30.1	(Ωm) 311.3 464.9 965.1 1237.8 1891.2
Spacing           0.5           1           2           5           10           20	(Ω) 99.1 74.0 76.8 39.4 30.1 15.2	(Ωm) 311.3 464.9 965.1 1237.8 1891.2 1905.0

P16177 - D	erreenacrinnig	Wind Farm
Date:	13/12/2016	
T2		
Easti	ng [ITM]	511134
North	ing [ITM]	551873
Elevation	[OD Malin]	394.3
	Direct	
Electrode	resistivity	Soil resistivity
Spacing	(Ω)	(Ωm)
0.5	158.8	498.9
1	128.3	806.1
2	92.1	1157.3
5	84.9	2667.1
10	29.9	1878.6
20	11.3	1420.0
40	5.2	1294.3

P16177 - Derreenacrinnig Wind Farm		
Date:	13/12/2016	
Т3		
Easting [ITM]		511370
Northing [ITM]		551919
Elevatior	[OD Malin]	397.5
	Direct	
Electrode	resistivity	Soil resistivity
Spacing	(Ω)	(Ωm)
0.5	59.2	186.0
		200.0
1	54.8	344.3
1	54.8 55.5	344.3 697.4
1 2 5	54.8 55.5 38.4	344.3 697.4 1206.3
1 2 5 10	54.8 55.5 38.4 24.8	344.3 697.4 1206.3 1558.2
1 2 5 10 20	54.8 55.5 38.4 24.8 16.3	344.3 697.4 1206.3 1558.2 2042.0

P16177 - D	erreenacrinnig	Wind Farm
Date:	14/12/2016	
T4		
Easti	ng [ITM]	511595
North	ing [ITM]	551958
Elevation	[OD Malin]	381.0
	Direct	
Electrode	resistivity	Soil resistivity
Spacing	(Ω)	(Ωm)
0.5	125.8	395.2
1	107.3	674.2
2	107.7	1353.4
5	48.5	1523.6
10	30.4	1910.0
20	21.4	2689.1
40	14.7	3686.9

P16177 - Derreenacrinnig Wind Farm		
Date: 14/12/2016		
T5		
Easting [ITM]		511853
North	ing [ITM]	552050
Elevation	[OD Malin]	359.7
	Direct	
Electrode	resistivity	Soil resistivity
Consider	$\langle \alpha \rangle$	
Spacing	(Ω)	(Ωm)
Spacing 0.5	(Ω) 242.0	(Ωm) 760.2
0.5	242.0 191.7	(Ωm) 760.2 1204.5
Spacing           0.5           1           2	(12) 242.0 191.7 106.6	(Ωm) 760.2 1204.5 1339.5
Spacing           0.5           1           2           5	(12) 242.0 191.7 106.6 46.2	(Ωm) 760.2 1204.5 1339.5 1451.4
Spacing           0.5           1           2           5           10	(12) 242.0 191.7 106.6 46.2 43.7	(Ωm) 760.2 1204.5 1339.5 1451.4 2745.7
Spacing           0.5           1           2           5           10           20	(12) 242.0 191.7 106.6 46.2 43.7 22.7	(Ωm) 760.2 1204.5 1339.5 1451.4 2745.7 2852.5

P101// - D	erreenacrinnig	Wind Farm
Date:	13/12/2016	
Т6		
Easti	511556	
North	ing [ITM]	552131
Elevation	[OD Malin]	374.3
	Direct	
Electrode	resistivity	Soil resistivity
Spacing	(Ω)	(Ωm)
05	235.0	738 3
0.5	255.0	750.5
0.3	159.8	1004.0
1 2	159.8 106.4	1004.0 1337.0
0.3 1 2 5	159.8 106.4 64.5	1004.0 1337.0 2026.3
0.3 1 2 5 10	159.8 106.4 64.5 21.3	1004.0 1337.0 2026.3 1338.3
1 2 5 10 20	159.8 106.4 64.5 21.3 11.7	1004.0 1337.0 2026.3 1338.3 1470.2

P16177 - Derreenacrinnig Wind Farm		
Date:	13/12/2016	
T7		
Easti	511768	
North	552187	
Elevation	[OD Malin]	358.1
Electrode Spacing	Direct resistivity (Ω)	Soil resistivity (Ωm)
0.5	119.2	374.5
1	127.5	801.1
2	74.7	938.7
5	39.0	1225.2
10	20.7	1300.6
20	10.2	1281 7
20	10.2	1201.7

P16177 - D	erreenacrinnig	Wind Farm
Date:	14/12/2016	
SB1 - Orientation North-South		
Easti	ng [ITM]	511144
North	ing [ITM]	551224
Elevatior	n [OD Malin]	241.7
	Direct	
Electrode	resistivity	Soil resistivity
	1-1	
Spacing	(Ω)	(Ωm)
Spacing 0.5	<b>(Ω)</b> 164.9	<b>(Ωm)</b> 518.0
Spacing 0.5 1	(Ω) 164.9 88.9	(Ωm) 518.0 558.6
Spacing           0.5           1           2	(Ω) 164.9 88.9 43.7	(Ωm) 518.0 558.6 549.1
Spacing           0.5           1           2           5	(Ω) 164.9 88.9 43.7 31.8	(Ωm) 518.0 558.6 549.1 999.0
Spacing           0.5           1           2           5           10	<ul> <li>(Ω)</li> <li>164.9</li> <li>88.9</li> <li>43.7</li> <li>31.8</li> <li>13.0</li> </ul>	(Ωm) 518.0 558.6 549.1 999.0 818.0
Spacing           0.5           1           2           5           10           20	(Ω)       164.9       88.9       43.7       31.8       13.0       14.0	(Ωm) 518.0 558.6 549.1 999.0 818.0 1759.2

P16177 - Derreenacrinnig Wind Farm					
Date:	14/12/2016				
SB2 - Orier	ntation North V	Vest-South East			
Easti	ng [ITM]	511132			
North	ing [ITM]	551210			
Elevatior	n [OD Malin]	241.6			
	Direct				
Electrode	resistivity	Soil resistivity			
Spacing	(Ω)	(Ωm)			
Spacing 0.5	(Ω) 311.0	<b>(Ωm)</b> 977.0			
<b>Spacing</b> 0.5 1	(Ω) 311.0 91.2	(Ωm) 977.0 573.0			
Spacing           0.5           1           2	<ul> <li>(Ω)</li> <li>311.0</li> <li>91.2</li> <li>122.2</li> </ul>	(Ωm) 977.0 573.0 1535.6			
Spacing           0.5           1           2           5	<ul> <li>(Ω)</li> <li>311.0</li> <li>91.2</li> <li>122.2</li> <li>45.6</li> </ul>	(Ωm) 977.0 573.0 1535.6 1432.5			
Spacing           0.5           1           2           5           10	(Ω) 311.0 91.2 122.2 45.6 23.3	(Ωm) 977.0 573.0 1535.6 1432.5 1463.9			
Spacing           0.5           1           2           5           10           20	<ul> <li>(Ω)</li> <li>311.0</li> <li>91.2</li> <li>122.2</li> <li>45.6</li> <li>23.3</li> <li>10.4</li> </ul>	(Ωm) 977.0 573.0 1535.6 1432.5 1463.9 1311.9			

P16177 - Derreenacrinnig Wind Farm						
Date:	14/12/2016					
SB3 - Orientation South West-North East						
Easti	ng [ITM]	511156				
North	ing [ITM]	551214				
Elevation	[OD Malin]	241.5				
	Direct					
Electrode	resistivity	Soil resistivity				
Spacing	(Ω)	(Ωm)				
0.5	416.0	1306.9				
1	248.0	1558.2				
2	106.0	1332.0				
5	30.4	955.0				
10	16.8	1057.4				
20	10.4	1300.6				
40	5.8	1447.6				

P16177 - Derreenacrinnig Wind Farm						
Date:	14/12/2016					
SB4 - Orier	ntation North W	/est-South East				
Easti	ng [ITM]	511157				
North	ing [ITM]	551239				
Elevation	[OD Malin]	242.2				
	Direct					
Electrode	resistivity	Soil resistivity				
Spacing	(Ω)	(Ωm)				
0.5	985.0	3094.4				
1	414.0	2601.2				
1	414.0 99.8	2601.2 1254.1				
1 2 5	414.0 99.8 78.8	2601.2 1254.1 2475.5				
1 2 5 10	414.0 99.8 78.8 32.4	2601.2 1254.1 2475.5 2035.7				
1 2 5 10 20	414.0 99.8 78.8 32.4 18.6	2601.2 1254.1 2475.5 2035.7 2337.3				

P16177 - Derreenacrinnig Wind Farm							
Date:	14/12/2016						
SB5 - Orier	SB5 - Orientation South West-North East						
Easti	ng [ITM]	511133					
North	ing [ITM]	551234					
Elevatior	[OD Malin]	243.6					
	Direct						
Electrode	resistivity	Soil resistivity					
	1 - 1						
Spacing	(Ω)	(Ωm)					
Spacing 0.5	(Ω) 100.2	(Ωm) 314.8					
Spacing 0.5 1	(Ω) 100.2 98.0	(Ωm) 314.8 615.7					
Spacing           0.5           1           2	(Ω) 100.2 98.0 77.3	(Ωm) 314.8 615.7 971.4					
Spacing           0.5           1           2           5	<ul> <li>(Ω)</li> <li>100.2</li> <li>98.0</li> <li>77.3</li> <li>49.3</li> </ul>	(Ωm) 314.8 615.7 971.4 1548.8					
Spacing           0.5           1           2           5           10	(Ω) 100.2 98.0 77.3 49.3 31.5	(Ωm) 314.8 615.7 971.4 1548.8 1979.1					
Spacing           0.5           1           2           5           10           20	<ul> <li>(Ω)</li> <li>100.2</li> <li>98.0</li> <li>77.3</li> <li>49.3</li> <li>31.5</li> <li>16.7</li> </ul>	(Ωm) 314.8 615.7 971.4 1548.8 1979.1 2093.5					

P16177 - Derreenacrinnig Wind Farm					
Date:	14/12/2016				
SB1 - Orier	ntation West-Ea	ast			
Easti	ng [ITM]	511144			
North	ing [ITM]	551224			
Elevatior	[OD Malin]	241.7			
	Direct				
Electrode	resistivity	Soil resistivity			
Spacing	( <b>0</b> )	(Om)			
Shacing	(22)	(32111)			
0.5	121.9	382.9			
0.5 1	121.9 111.5	382.9 700.6			
0.5 1 2	121.9 111.5 46.1	382.9 700.6 579.3			
0.5 1 2 5	121.9 111.5 46.1 20.1	382.9 700.6 579.3 631.4			
0.5 1 2 5 10	121.9 111.5 46.1 20.1 14.6	382.9 700.6 579.3 631.4 916.7			
0.5 1 2 5 10 20	121.9 111.5 46.1 20.1 14.6 11.5	382.9 700.6 579.3 631.4 916.7 1445.1			

## **APPENDIX C: SEISMIC PROFILE SOUNDING RESULTS**

#### P-Wave Geophysical Interpretation Summary Sheets

Below are the results from Seismic Refraction (Vp) and generated MASW (Vs) data combined with the engineering moduli. A geophysical interpretation and estimated excavatability has been included with the results.

 Position:
 S1

 Easting:
 510914.7

 Northing:
 551850.4

 Elevation
 373.4

Depth [m	bgl] To	P Wave [m/s]	S Wave	Density ② [kg/m³]	Poissons Ratio (3)	K Bulk Modulus [MPa]	E Youngs Modulus [MPa]	Small Strain Shear Modulus [MPa]	Geophysical Interpretation	Excavatability
	10	[, 5]	[, 5]	["6/ "]	U	[[[[[	[	[[[[[	ecopitysical interpretation	Executationity
0	0.4	409	136	1394	0.44	198.67	74.50	25.91	Peat	Diggable
0.4	1	968	323	1729	0.44	1380.27	517.58	180.03	Highly fractured SILTSTONE	Probable Rippable
1	1.5	1529	510	1938	0.44	3860.66	1447.68	503.54	Fractured SILTSTONE	Probable Rippable
1.5	2.4	2194	731	2122	0.44	8700.16	3262.42	1134.75	Slightly fractured SILTSTONE	Marginal Rippable
2.4	-	3480	1160	2381	0.44	24563.94	9211.08	3203.85	Fresh SILTSTONE	Non Rippable

① S Wave estimated from p-wave velocity on a 1:3 ratio

(2) Density computed from p-wave velocity

Position:	S2
Easting:	511134.3
Northing:	551873.3
Elevation	394.3

Depth [m	bgl]	P Wave	S Wave	Density 2	Poissons Ratio	K Bulk Modulus	E Youngs Modulus	Small Strain Shear Modulus		
From	То	[m/s]	[m/s]	[kg/m³]	3	[MPa]	[MPa]	[MPa]	Geophysical Interpretation	Excavatability
0	0.4	837	279	1667	0.44	995.12	373.16	129.79	Highly fractured SANDSTONE	Probable Rippable
0.4	1.3	1464	488	1918	0.44	3501.16	1312.88	456.65	Fractured SANDSTONE	Probable Rippable
1.3	1.9	2006	669	2075	0.44	7111.96	2666.87	927.61	Slightly fractured SANDSTONE	Probable Rippable
1.9	2.6	2653	884	2225	0.44	13339.93	5002.26	1739.91	Slightly fractured SANDSTONE	Marginal Rippable
2.6	-	3748	1249	2426	0.44	29026.44	10884.44	3785.89	Fresh SANDSTONE	Non Rippable

1 S Wave estimated from p-wave velocity on a 1:3 ratio

(2) Density computed from p-wave velocity

Position:	S3
Easting:	511370.5
Northing:	551918.1
Elevation	397.5

Depth [m	bgl]	P Wave	S Wave	Density ②	Poissons Ratio	K Bulk Modulus	E Youngs Modulus	Small Strain Shear Modulus		
From	То	[m/s]	[m/s]	[kg/m³]	3	[MPa]	[MPa]	[MPa]	Geophysical Interpretation	Excavatability
0	0.4	409	136	1394	0.44	198.67	74.50	25.91	Peat	Diggable
0.4	1	968	323	1729	0.44	1380.27	517.58	180.03	Firm Overburden	Diggable
1	1.5	1529	510	1938	0.44	3860.66	1447.68	503.54	Fractured SILTSTONE	Probable Rippable
1.5	2.4	2194	731	2122	0.44	8700.16	3262.42	1134.75	Slightly fractured SILTSTONE	Marginal Rippable
2.4	-	3480	1160	2381	0.44	24563.94	9211.08	3203.85	Fresh SILTSTONE	Non Rippable

① S Wave estimated from p-wave velocity on a 1:3 ratio

(2) Density computed from p-wave velocity

Position:	<b>S4</b>									
Easting:	511591.9									
Northing:	551961.0									
<b>Elevation</b>	381.0									
			S Wave	Density	Poissons	K Bulk Modulus	E Youngs Modulus	Small Strain Shear Modulus		
Depth [m l	bgl] To	P Wave	(1) [m/s]	(Z) [kg/m³]		[MPa]	[MPa]	[MPa]	Geophysical Interpretation	Excavatability
Depth [m l From	bgl] To	P Wave [m/s]	( <u>1</u> ) [m/s]	[kg/m ³ ]	Ratio 3	[MPa]	[MPa]	[MPa]	Geophysical Interpretation	Excavatability
Depth [m   From 0	<b>bgl] To</b> 0.4	P Wave [m/s] 1681	[m/s] 560	[kg/m ³ ]	<b>Ratio</b> <b>3</b> 0.44	[MPa] 4778.28	[MPa] 1791.78	[MPa] 623.23	Geophysical Interpretation Fractured SILTSTONE	Excavatability Probable Rippable
Depth [m   From 0 0.4	<b>5gl]</b> <b>To</b> 0.4 0.7	P Wave [m/s] 1681 2798	[m/s] 560 933	(2) [kg/m ³ ] 1985 2255	<b>Ratio</b> 3 0.44 0.44	[MPa] 4778.28 15036.68	[MPa] 1791.78 5638.51	[MPa] 623.23 1961.22	Geophysical Interpretation Fractured SILTSTONE Slightly fractured SILTSTONE	Excavatability Probable Rippable Marginal Rippable
Depth [m   From 0.4 0.7	bgl]           To           0.4           0.7           1	P Wave [m/s] 1681 2798 3473	[m/s] 560 933 1158	[kg/m ³ ] 1985 2255 2380	0.44 0.44 0.44	[MPa] 4778.28 15036.68 24452.90	[MPa] 1791.78 5638.51 9169.44	[MPa] 623.23 1961.22 3189.37	Geophysical Interpretation Fractured SILTSTONE Slightly fractured SILTSTONE Fresh SILTSTONE	Excavatability Probable Rippable Marginal Rippable Non Rippable

1 S Wave estimated from p-wave velocity on a 1:3 ratio

(2) Density computed from p-wave velocity

Position:	S5
Easting:	511852.2
Northing:	552050.2
Elevation	359.7

Depth [m l	ogl]	P Wave	S Wave	Density ②	Poissons Ratio	K Bulk Modulus	E Youngs Modulus	Small Strain Shear Modulus		
From	То	[m/s]	[m/s]	[kg/m³]	3	[MPa]	[MPa]	[MPa]	Geophysical Interpretation	Excavatability
0	0.4	1088	363	1780	0.44	1795.39	673.24	234.17	Highly fractured SILTSTONE	Probable Rippable
0.4	0.9	2031	677	2081	0.44	7312.94	2742.23	953.82	Slightly fractured SILTSTONE	Probable Rippable
0.9	2.3	2641	880	2222	0.44	13204.55	4951.49	1722.26	Slightly fractured SILTSTONE	Marginal Rippable
2.3	2.8	2740	913	2243	0.44	14344.43	5378.93	1870.93	Fresh SILTSTONE	Marginal Rippable
2.8	-	3269	1090	2344	0.44	21339.20	8001.85	2783.25	Fresh SILTSTONE	Non Rippable

① S Wave estimated from p-wave velocity on a 1:3 ratio

(2) Density computed from p-wave velocity

Position:	S6			
Easting:	511557.4			
Northing:	552129.5			
Elevation	374.3			

Depth [m l	ogl]	P Wave	S Wave	Density ②	Poissons Ratio	K Bulk Modulus	E Youngs Modulus	Small Strain Shear Modulus		
From	То	[m/s]	[m/s]	[kg/m³]	3	[MPa]	[MPa]	[MPa]	Geophysical Interpretation	Excavatability
0	0.4	959	320	1725	0.44	1351.56	506.81	176.28	Highly fractured SILTSTONE	Probable Rippable
0.4	0.7	1792	597	2017	0.44	5517.66	2069.03	719.66	Fractured SILTSTONE	Probable Rippable
0.7	1	2279	760	2142	0.44	9476.97	3553.71	1236.07	Slightly fractured SILTSTONE	Probable Rippable
1	1.3	2810	937	2257	0.44	15182.17	5693.07	1980.20	Fresh SILTSTONE	Marginal Rippable
1.3	1.5	3310	1103	2351	0.44	21946.11	8229.44	2862.41	Fresh SILTSTONE	Non Rippable
1.5	-	3736	1245	2424	0.44	28817.75	10806.19	3758.67	Fresh SILTSTONE	Non Rippable

1 S Wave estimated from p-wave velocity on a 1:3 ratio

(2) Density computed from p-wave velocity

Position:	S7			
Easting:	511769.3			
Northing:	552190.0			
Elevation	347.5			

Depth [m bgl]		P Wave	S Wave	Density ②	Poissons Ratio	K Bulk Modulus	E Youngs Modulus	Small Strain Shear Modulus		
From	То	[m/s]	[m/s]	[kg/m³]	3	[MPa]	[MPa]	[MPa]	Geophysical Interpretation	Excavatability
0	0.7	563	188	1510	0.44	407.74	152.90	53.18	Firm OVERBURDEN	Diggable
0.7	1.2	1236	412	1838	0.44	2392.14	897.01	312.00	Fractured SILTSTONE	Probable Rippable
1.2	1.8	1816	605	2024	0.44	5685.33	2131.90	741.53	Fractured SILTSTONE	Probable Rippable
1.8	2.4	2407	802	2171	0.44	10716.82	4018.63	1397.79	Slightly fractured SILTSTONE	Marginal Rippable
2.4	-	3480	1160	2381	0.44	24563.94	9211.08	3203.85	Fresh SILTSTONE	Non Rippable

① S Wave estimated from p-wave velocity on a 1:3 ratio

(2) Density computed from p-wave velocity

# **APPENDIX D: SEISMIC REFRACTION PROFILE CROSS SECTIONS**

All profiles are displayed from west towards east. The interpreted top of rock has been given as a thick black line on the cross sections. See APPENDIX A for location map of all profiles collected.



Figure AB.1: P-Wave Seismic Velocity Cross Section for Turbine Base T1.

P16177_Gp_Rp_F01

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Figure AB.2: P-Wave Seismic Velocity Cross Section for Turbine Base T2.



Figure AB.3: P-Wave Seismic Velocity Cross Section for Turbine Base T3.



Figure AB.4: P-Wave Seismic Velocity Cross Section for Turbine Base T4.



Figure AB.5: P-Wave Seismic Velocity Cross Section for Turbine Base T5.



Figure AB.6: P-Wave Seismic Velocity Cross Section for Turbine Base T6.

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Figure AB.7: P-Wave Seismic Velocity Cross Section for Turbine Base T7.

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# APPENDIX C

# LABORATORY RESULTS

Natural Moisture Content Atterberg Limits Particle Size Distribution (grading) pН SO4 water soluble SO4 acid soluble Loss on ignition Proctor compaction (Moisture content/dry density relationship) California Bearing Ratio, CBR CBR Moisture content relationship UCS with Young's Modulus and Poisson's Ratio Point Load Test (IP50) Magnesium sulphate soundness value MSSV, pН SO₄ water soluble
## **KEY TO SYMBOLS - LABORATORY TEST RESULT**

U	Undisturbed Sample	
P	Piston Sample	
TWS	Thin Wall Sample	
В	Bulk Sample - Disturbed	
D	Jar Sample - Disturbed	
W	Water Sample	
ρΗα	Acidity/Alkalinity Index	
SO3	% - Total Sulphate Content (acid soluble)	
SO ₃	g/ltr - Water Soluble Sulphate (Water or 2:1 Aqueo	ous Soil Extract)
+	Calcareous Reaction	,
CI	Chloride Content	
PI	Plasticity Index	
<425	% of material in sample passing 425 micron sieve	
LL	Liquid Limit	
PL	Plastic Limit	
MC	Water Content	
NP	Non Plastic	
Yb	Bulk Density	
Yd	Dry Density	
Ps	Particle Density	
U/D	Undrained/Drained Triaxial	
U/C	Unconsolidated/Consolidated Triaxial	
T/M	Single Stage/Multistage Triaxial	
100/38	Sample Diameter (mm)	
REM	Remoulded Triaxial Test Specimen	
TST	Triaxial Suction Test	
V	Vane Test	
DSB	Drained Shear Box	
RSB	Residual Shear Box	
RS	Ring Shear	
$\sigma_3$	Cell Pressure	
$\sigma_1 - \sigma_3$	Deviator Stress	
С		
c_	Effective Conesion Intercept	
φ	Angle of Shearing Resistance - Degrees	
φ_ cf	Strain of Engline	
٤١ *	Stiall at Fallule	
**	Failed under 2 nd Load	
#		
# ##	Excessive Strain	
" <i>"</i>	Effective Overburden Pressure	
р_0 m.	Coefficient of Volume Decrease	
C,	Coefficient of Consolidation	
Opt	Optimum	
Nat	Natural	
Std	Standard Compaction - 2.5kg Rammer	(¶ CBR)
Hvv	Heavy Compaction - 4.5kg Rammer	(§ CBR)
Vib	Vibratory Compaction	
CBR	California Bearing Ratio	
Sat m.c.	Saturation Moisture Content	
MCV	Moisture Condition Value	
	1 1	

Key sheet



	Natural Moisture Content/Atterberg Limits Summary	Job Ref
GEOTECHNICAL	BS 1377 : Part 2 : 1990 : Clause 3	
Location	Dereenacrinnig Windfarm	P16177

Hole ID	Sample Ref	Depth (m)	Sample Type	Sample Description	мс	LL	PL	PI	% Pass 425
TP01	1	0.3	В	Silty sandy GRAVEL with medium cobble content		84	61	23	39.3
TP01	2	0.3	D	Silty sandy GRAVEL	18				
TP02	3	0.9	В	Very silty very sandy GRAVEL with low cobble content		32	24	8	46.1
TP02	4	0.9	D	Very silty very sandy GRAVEL	10				
TP03	4	0.4	D	Slightly sandy gravelly SILT	16				
TP03	5	1.4	В	Silty very sandy GRAVEL with medium cobble content		32	23	9	40.1
TP03	6	1.4	D	Silty very sandy GRAVEL	9.2				
TP04	2	0.4	D	Slightly gravelly sandy SILT	9.6				
TP04	3	1	В	Silty very sandy GRAVEL with medium cobble content		34	26	8	40
TP04	4	1	D	Slightly sandy gravelly SILT	28				
TP05	1	0.4	В	Slightly sandy gravelly SILT with high cobble content		187	102	85	49.5
TP05	2	0.4	D	Slightly sandy gravelly SILT	38				
TP06	2	0.4	D	Slightly gravelly sandy SILT	45				
TP06	3	0.75	В	Slightly sandy slightly gravelly SILT		29	NP	NP	62.2
TP06	4	0.75	В	Slightly sandy slightly gravelly SILT	15				
TP07	2	0.6	D	Slightly gravelly sandy SILT	15				
TP07	4	1.3	D	Slightly gravelly sandy SILT	32				
TP08	2	0.5	D	Slightly sandy slightly gravelly SILT	248				
TP08	4	0.8	В	Very silty very sandy GRAVEL with low cobble content	52	34	24	10	51.4
TP09	3	0.4	В	Slightly sandy slightly gravelly SILT	14				
TP10	4	0.9	D	Slightly sandy gravelly SILT	11				
TP11	2	0.4	D	Slightly sandy slightly gravelly SILT	32				

	Natural Moisture Content/Atterberg Limits Summary	Job Ref
GEOTECHNICAL	BS 1377 : Part 2 : 1990 : Clause 3	
Location	Dereenacrinnig Windfarm	P16177

Hole ID	Sample Ref	Depth (m)	Sample Type	Sample Description	мс	LL	PL	PI	% Pass 425
TP11	4	0.9	D	Slightly gravelly sandy SILT	11				
TP12	2	1	В	PEAT	765				
TP13	4	0.35	D	Slightly sandy gravelly SILT	25				
TP14	2	0.5	D	Slightly sandy slightly gravelly SILT	20				
TP15	2	0.5	D	Slightly sandy gravelly SILT	36				
TP15	4	1.1	D	Slightly sandy gravelly SILT	15				
TP15	6	2.1	D	Slightly sandy gravelly SILT	12				
TP16	2	0.3	D	Slightly sandy slightly gravelly SILT	18				
TP17	2	0.25	D	Slightly sandy slightly gravelly SILT	27				
TP18	2	0.5	D	Slightly sandy gravelly SILT	38				-
TP18	3	0.8	В	Slightly sandy gravelly SILT with medium cobble content		31	24	7	62.3
TP18	4	0.8	D	Slightly sandy gravelly SILT	12				
TP19	2	0.9	D	Slightly sandy gravelly SILT	39				
TP20	2	0.55	D	Slightly sandy gravelly SILT	20				
ТРОЗА	2	0.6	D	Slightly gravelly sandy SILT	28				
ТР03А	4	1.3	D	Slightly sandy gravelly SILT	12				
TP06A	2	0.2	D	Slightly sandy slightly gravelly SILT	18				
TP07A	2	0.3	D	Slightly sandy gravelly SILT	15				
TP15A	4	0.8	D	Slightly sandy gravelly SILT	12				
TPS1	1	0.55	В	Silty sandy GRAVEL with medium cobble content		30	23	7	46.6
TPS1	2	0.55	D	Silty sandy GRAVEL	13				
TPS1	4	1.5	D	Silty sandy GRAVEL	13				i

■—— PRIORITY GEOTECHNICAL	Natural Moisture Content/Atterberg Limits Summary BS 1377 : Part 2 : 1990 : Clause 3	Job Ref
Location	Dereenacrinnig Windfarm	P16177

Hole ID	Sample Ref	Depth (m)	Sample Type	Sample Description	мс	LL	PL	PI	% Pass 425
TPS2	4	1	D	Slightly sandy gravelly SILT					
TPS3	1	0.5	В	Slightly sandy GRAVEL with high cobble content		60	38	22	57.9
TPS3	2	0.5	D	Slightly sandy GRAVEL 42					
TPS3	3	1.1	B	Slightly sandy gravelly SILT low cobble content 26		21	5	65.2	
TPS3	4	1.1	D	Slightly sandy gravelly SILT	21				



Sievii	ng	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	89		
63	89		
50	80		
37.5	72		
28	63		
20	59		
14	44		
10	38		
6.3	31		
5	28		
3.35	25		
2	21		
1.18	17		
0.6	14		
0.425	12		
0.3	11		
0.212	10		
0.15	9		
0.063	7		

Test Method					
BS 1377 : Part 2 : 1990					
Sieving	Clause 9.2				
Sedimentation	N/A				

Sample Proportions					
Cobbles	13.5				
Gravel	65.6				
Sand	14.1				
Silt & Clay	6.7				

Grading Analy	/sis
D100	90.000
D60	22.186
D10	0.217
Uniformity Coefficient	102











1.18

0.6

0.425

0.3

0.212

0.15

0.063

65

60

57

55

53

49

37

Grading Analysis				
D100	50.000			
D60	0.646			
D10				
Uniformity Coefficient	N/A			











0.6

0.425

0.3

0.212

0.15

0.063

42

41

39

38

36

31

Grading Analysis		
D100	90.000	
D60	8.036	
D10		
Uniformity Coefficient	N/A	



Siev	ing	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	96		
50	85		
37.5	82		
28	76		
20	70		
14	63		
10	57		
6.3	53		
5	50		
3.35	47		
2	43		
1.18	39		
0.6	34		
0.425	32		
0.3	30		
0.212	28		
0.15	27		
0.063	23		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.2	
Sedimentation	N/A	

Sample Proportions		
Cobbles	6.8	
Gravel	50.5	
Sand	20.2	
Silt & Clay	22.5	

Grading Analysis		
D100	75.000	
D60	11.864	
D10		
Uniformity Coefficient	N/A	



Sieving		Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	87		
75	84		
63	81		
50	80		
37.5	71		
28	71		
20	65		
14	60		
10	55		
6.3	47		
5	45		
3.35	40		
2	35	1	
1.18	21		
0.6	20		
0.425	19		
0.3	19		
0.212	18		
0.15	18		
0.063	18		

lest wethod		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.2	
Sedimentation	N/A	

Sample Proportions		
Cobbles	19.0	
Gravel	45.9	
Sand	17.4	
Silt & Clay	17.8	

Grading Analysis	
D100	125.000
D60	13.664
D10	
Uniformity Coefficient	N/A



	Sievii	ng	Sedimen	tation
	Particle Size mm	% Passing	Particle Size mm	% Passing
	125	100		
	90	90		
	75	90		
	63	76		
	50	76		
	37.5	67		
	28	56		
	20	44		
	14	35		
	10	31		
	6.3	26		
	5	24		
	3.35	23		
	2	21		
	1.18	20		
	0.6	19		
Î	0.425	18		
	0.3	18		
	0.212	17		
	0.15	17		
	0.063	16		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving Clause 9.2		
Sedimentation	N/A	

Sample Proportions		
Cobbles	24.5	
Gravel	54.2	
Sand	5.6	
Silt & Clay	15.7	

Grading Analysis				
D100	125.000			
D60	31.307			
D10				
Uniformity Coefficient	N/A			



Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	99		
50	99		
37.5	94		
28	92		
20	88		r
14	85		
10	80		
6.3	73		
5	71		
3.35	66		
2	60		
1.18	56		
0.6	52		
0.425	50		
0.3	49		
0.212	48		
0.15	47		
0.063	44		

Sample Proportions				
Cobbles	1.5			
Gravel	38.5			
Sand	15.9			
Silt & Clay	44.1			

Grading Analysis				
D100	75.000			
D60	1.999			
D10				
Uniformity Coefficient	N/A			





	Remarks	



	Remarks	
		1







rest Conditions				
Sample Retained on 20 mm sieve	%	2.3		
Seating Load - TOP	N			
Seating Load - BASE	N			
Surcharge	kg	8		

Accepted CBR	65.6	51.6
5.0	65.6	49.2
2.5	63.1	51.6
	TOP	BASE

Remarks	



	Remarks	······································









	Method of Compation				
ration	Rammer compaction with specified effort				
eba	Hammer type 2.5kg Rammer				
ď	Soaking Period days				
	Amount of Swell mm				

Sample Conditions				
Natural Moisture Content	%	15		
Moisture Content - TOP	%	15		
Moisture Content - BASE	%	13		
Bulk Density	Mg/m³	2.21		
Dry Density	Mg/m³	1.92		

Test Conditions			
Sample Retained on 20 mm sieve	%	13.3	
Seating Load - TOP	N		
Seating Load - BASE	N		
Surcharge	kg	8	

Ponotration mm	CBR Values %	
Penetration mm	TOP	BASE
2.5	0.5	0.5
5.0	0.6	0.5
Accepted CBR	0.6	0.5

	Remarks	





ration	Rammer compaction with specified effort				
гера	Hammer type		2.5kg Rammer		
בֿ	Soaking Period	lays			
	Amount of Swell	mm			

Sample Conditions				
Natural Moisture Content % 17				
Moisture Content - TOP	%	17		
Moisture Content - BASE	%	15		
Bulk Density	Mg/m³	2.19		
Dry Density	Mg/m³	1.87		

Test Condition	s	
Sample Retained on 20 mm sieve	%	15.6
Seating Load - TOP	N	
Seating Load - BASE	N	
Surcharge	kg	8

Accepted CBR	0.9	0.6
5.0	0.9	0.6
2.5	0.7	0.5
Penetration mm	ТОР	BASE
	CBR Values %	

	Remarks	



Test Condition	s	
Sample Retained on 20 mm sieve	%	31.3
Seating Load - TOP	N	
Seating Load - BASE	N	
Surcharge	kg	8

5.0	0,7	0.9
2.5	0.5	0.5
	TOP	BASE
Penetration mm	CBR Values %	

	Remarks	



	Method of					
ration	Rammer compaction with specified effort					
repa	Hammer type 2.5kg Ramme					
١	Soaking Period days					
	Amount of Swell mm					

Sample C	onditions	
Natural Moisture Content	%	37
Moisture Content - TOP	%	37
Moisture Content - BASE	%	35
Bulk Density	Mg/m³	1.82
Dry Density	Mg/m³	1.33

Test Conditions			
Sample Retained on 20 mm sieve	%	10.0	
Seating Load - TOP	N		
Seating Load - BASE	N		
Surcharge	kg	8	

Popotration mm	CBR Values %	
	ТОР	BASE
2.5	0.3	0.3
5.0	0.4	0.3
Accepted CBR	0.4	0.3

	Remarks	
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L	1	



Test Conditions				
Sample Retained on 20 mm sieve	%	9.3		
Seating Load - TOP	N			
Seating Load - BASE	N			
Surcharge	kg	8		

Donotration mm	CBR Values %	
Fenetration mm	TOP	BASE
2.5	0.1	0.2
5.0	0.2	0.3
Accepted CBR	0.2	0.3

			Remarks	
Ì				
		[		



Test Conditions				
Sample Retained on 20 mm sieve	%	15.7		
Seating Load - TOP	N			
Seating Load - BASE	N			
Surcharge	kg	8		

Benetration mm	CBR Values %	
Penetration mm	TOP	BASE
2.5	0.2	0.1
5.0	0.2	0.1
Accepted CBR	0.2	0.1

			Remarks	
L		I		


eparation	Method of Compation		
	Rammer compaction with specified effort		
	Hammer type	2.5kg Rammer	
ā	Soaking Period days		
	Amount of Swell mm		

Sample C	onditions	
Natural Moisture Content	%	36
Moisture Content - TOP	%	36
Moisture Content - BASE	%	31
Bulk Density	Mg/m³	1.82
Dry Density	Mg/m³	1.34

Test Condition	S	
Sample Retained on 20 mm sieve	%	24.1
Seating Load - TOP	N	
Seating Load - BASE	N	
Surcharge	kg	8

Ponetration mm	CBR Values %		
Penetration mm	TOP	BASE	
2.5	0.3	0.4	
5.0	0.4	0.4	
Accepted CBR	0.4	0.4	

	Remarks	



		Remarks	
1			
	 		į



reparation	Rammer compaction with specified effort				
	Hammer type		2.5kg Rammer		
۵	Soaking Period	days			
	Amount of Swell	mm			

Sample C	onditions	
Natural Moisture Content	%	49
Moisture Content - TOP	%	49
Moisture Content - BASE	%	50
Bulk Density	Mg/m³	1.73
Dry Density	Mg/m³	1.16

Test Conditions			
Sample Retained on 20 mm sieve	%	28.1	
Seating Load - TOP	N		
Seating Load - BASE	N		
Surcharge	kg	8	

Accepted CBR	0.2	0.2	
5.0	0.2	0.2	
2.5	0.2	0.2	
Penetration mm	TOP	BASE	
D	CBR Values %		

Remarks	······································



Test Condition	s		
Sample Retained on 20 mm sieve	%	10.4	_
Seating Load - TOP	N		
Seating Load - BASE	N		
Surcharge	kg	8	_

Accepted CBR	1.1	0.8	
5.0	1.1	0.8	
2.5	0.8	0.5	
Penetration min	TOP	BASE	
Ponotration mm	CBR Values %		

 	Remarks	



	Method of Compation						
ration	Rammer compaction with specif	ied effort					
repa	2.5kg Rammer						
ā	G. Soaking Period days						
	Amount of Swell mm						

Sample Conditions					
Natural Moisture Content	%	14			
Moisture Content - TOP	%	14			
Moisture Content - BASE	%	11			
Bulk Density	Mg/m³	2.01			
Dry Density	Mg/m³	1.76			

Test Conditions					
Sample Retained on 20 mm sieve	%	33.0			
Seating Load - TOP	N				
Seating Load - BASE	N				
Surcharge	kg	9			

Panatration mm	CBR Values %			
renetiation min	ТОР	BASE		
2.5	0.6	2.1		
5.0	1.1	2.4		
Accepted CBR	1.1	2.4		

		Remarks	
			:
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Chemtest Ltd. The right chemistry to deliver results Chemtest Ltd. Depot Road Newmarket CB8 0AL Tel: 01638 606070 Email: info@chemtest.co.uk

Report No.:	17-02390-1		
Initial Date of Issue:	03-Feb-2017		
Client	Priority Geotechnical Ltd		
Client Address:	Unit 12 Owenacurra Business Park Midleton County Cork Ireland		
Contact(s):	Colette Kelly		
Project	P16177 Derreenacrinnig Wind Farm		
Quotation No.:		Date Received:	01-Feb-2017
Order No.:	9262	Date Instructed:	01-Feb-2017
No. of Samples:	3		
Turnaround (Wkdays):	3	Results Due:	03-Feb-2017
Date Approved:	03-Feb-2017		
Approved By:			
Mandaley			
Details:	Glynn Harvey, Laboratory Manager		



# Results - Soil

Client: Priority Geotechnical Ltd	Chemtest Job No.:			17-02390	17-02390	17-02390	
Quotation No.:	Chemtest Sample ID.:		406144	406145	406146		
	Client Sample ID.:			TP19	TPS1	TPS3	
		Sample Type:			SOIL	SOIL	SOIL
		Top Depth (m):			0.9	0.55	1.1
		Date Sampled:		31-Jan-2017	31-Jan-2017	31-Jan-2017	
Determinand	Accred.	SOP	Units	LOD			
Moisture	Ν	2030	%	0.020	27	9.9	9.3
рН	U	2010		N/A		7.8	
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	0.010		0.013	
Sulphate (Acid Soluble)	U	2430	%	0.010		< 0.010	
LOI	U	2610	%	0.10	5.1	0.86	0.73



# **Test Methods**

SOP	Title	Parameters included	Method summary
2010	pH Value of Soils	рН	pH Meter
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES
2430	Total Sulphate in soils	Total Sulphate	Acid digestion followed by determination of sulphate in extract by ICP-OES.
2610	Loss on Ignition	loss on ignition (LOI)	Determination of the proportion by mass that is lost from a soil by ignition at 550°C.



## **Report Information**

#### Key

- U UKAS accredited
- M MCERTS and UKAS accredited
- N Unaccredited
- S This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
- SN This analysis has been subcontracted to a UKAS accredited laboratory that is not accredited for this analysis
- T This analysis has been subcontracted to an unaccredited laboratory
- I/S Insufficient Sample
- U/S Unsuitable Sample
- N/E not evaluated
- < "less than"
- > "greater than"

Comments or interpretations are beyond the scope of UKAS accreditation The results relate only to the items tested Uncertainty of measurement for the determinands tested are available upon request None of the results in this report have been recovery corrected All results are expressed on a dry weight basis The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols For all other tests the samples were dried at < 37°C prior to analysis All Asbestos testing is performed at the indicated laboratory Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

#### **Sample Deviation Codes**

- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container

#### **Sample Retention and Disposal**

All soil samples will be retained for a period of 45 days from the date of receipt All water samples will be retained for 14 days from the date of receipt Charges may apply to extended sample storage

#### If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.co.uk</u>



Chemtest Ltd. The right chemistry to deliver results Chemtest Ltd. Depot Road Newmarket CB8 0AL Tel: 01638 606070 Email: info@chemtest.co.uk

Report No.:	17-01935-1		
Initial Date of Issue:	30-Jan-2017		
Client	Priority Geotechnical Ltd		
Client Address:	Unit 12 Owenacurra Business Park Midleton County Cork Ireland		
Contact(s):	Colette Kelly		
Project	P16177 Derreenacrinn in WF		
Quotation No.:		Date Received:	26-Jan-2017
Order No.:	9262	Date Instructed:	26-Jan-2017
No. of Samples:	3		
Turnaround (Wkdays):	3	Results Due:	30-Jan-2017
Date Approved:	30-Jan-2017		
Approved By:			
Mandal			
Details:	Glynn Harvey, Laboratory Manager		



# Results - Soil

Client: Priority Geotechnical Ltd	Chemtest Job No.:			17-01935	17-01935	17-01935	
Quotation No.:	Chemtest Sample ID.:			404041	404042	404043	
Order No.: 9262	Client Sample Ref .:		TP15	TP3A	TP12		
	Sample Type:			SOIL	SOIL	SOIL	
	Top Depth (m):			2.1	1.3	1.0	
	Date Sampled:		24-Jan-2017	24-Jan-2017	24-Jan-2017		
Determinand	Accred.	SOP	Units	LOD			
Moisture	N	2030	%	0.020	8.5	11	93
рН	U	2010		N/A	8.1	7.4	
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	0.010	< 0.010	< 0.010	
Sulphate (Acid Soluble)	U	2430	%	0.010	< 0.010	< 0.010	
LOI	U	2610	%	0.10			81



# **Test Methods**

SOP	Title	Parameters included	Method summary
2010	pH Value of Soils	рН	pH Meter
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES
2430	Total Sulphate in soils	Total Sulphate	Acid digestion followed by determination of sulphate in extract by ICP-OES.
2610	Loss on Ignition	loss on ignition (LOI)	Determination of the proportion by mass that is lost from a soil by ignition at 550°C.



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- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
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#### If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.co.uk</u>



Chemtest Ltd. The right chemistry to deliver results Chemtest Ltd. Depot Road Newmarket CB8 0AL Tel: 01638 606070 Email: info@chemtest.co.uk

Report No.:	17-01080-1		
Initial Date of Issue:	20-Jan-2017		
Client	Priority Geotechnical Ltd		
Client Address:	Unit 12 Owenacurra Business Park Midleton County Cork Ireland		
Contact(s):	Colette Kelly		
Project	P16177 - Derreenacrinnin WF		
Quotation No.:		Date Received:	17-Jan-2017
Order No.:	9296	Date Instructed:	17-Jan-2017
No. of Samples:	1		
Turnaround (Wkdays):	5	Results Due:	23-Jan-2017
Date Approved:	20-Jan-2017		
Approved By:			
Kh.			
Details:	Robert Monk, Technical Development Chemist		



Client: Priority Geotechnical Ltd		Che	mtest Jo	ob No.:	17-01080
Quotation No.:	(	Chemte	st Sam	ple ID.:	400067
Order No.: 9296		Clie	nt Samp	le Ref.:	BHT01
			Sampl	e Type:	SOIL
			oth (m):	5.80	
	Date Sampled:				13-Jan-2017
Determinand	Accred.	SOP	Units	LOD	
Moisture	N	2030	%	0.020	1.5
рН	U	2010		N/A	8.7
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	0.010	< 0.010



# Test Methods

SOP	Title	Parameters included	Method summary
2010	pH Value of Soils	рН	pH Meter
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES



## **Report Information**

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#### **Sample Deviation Codes**

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#### If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.co.uk</u>

Prior	ity Go	eote	chn	ical L	.imit	ed	<b>⊒</b> —(?)→	Test Type D - Diametral, /	A - Axial, I - Irregu	ular Lump							Ро	int Load	l Test Results	
Proje	ct	N6 GC	TP Pł	nase 3				Direction (U = Par - parallel to	<ul> <li>unknown or ran</li> <li>planes of weakn</li> </ul>	dom) iess				Diametral Axial			E	Block/irregular lump		
Proje	ct No	P1618	5					Per - perpendicular to planes of weakness Dimensions Dps - Distance between platens ( platen separation ) Dps' - at failure				↓ P								
Carried	d out by				06/	GH 01/2017	]	Lne - Length fro W - Width of s Machine Ra	om platens to nea shortest dimensio am Area, cm ²	arest free end on perpendicular	to load, P		D _{ps} ↓	↓		w W		W	Dps	
rehole	ple Top, BGL	ıple Ref	ole Type	ample ase, BGL	ecimen Ref	ecimen epth, BGL	Description	Test see Fig 5	Type ISRM and 8	ire Valid Y/N)		Dimensions	6	Gauge	P Failure Load,	De equivalent diameter	ls MPa	ls(50) point load index,	Remarks	
Bo	Sam m	Sam	Sam	ы С С С С С	Spe	а С С С С		Type (D, A, I)	Direction (Par/Per/U)	Failu (	L mm	Dps, mm	W mm	reading, Kiv	kN	mm	WI a	MPa		
BHT01	3.0						SILTSTONE	D	Par	Y	97	76	76	3.28	3.28	76.0	0.57	0.69	Undulating Rough	
BHT01	5.2						SILTSTONE	D	Par	Y	169	76	76	19.6	19.60	76.0	3.39	4.10	Undulating Rough	
BHT02	5						SANDSSTONE	D	Par	Y	130	76	76	22.43	22.43	76.0	3.88	4.69	Undulating Rough	
BHT03	3.15						SILTSTONE	I	Par	Y	40	69	76	23.72	23.72	81.7	3.55	4.43	Undulating Rough	
BHT03	5.35						SANDSTONE	D	Par	Y	85	76	76	5.05	5.05	76.0	0.87	1.06	Undulating Rough	
BHT04	4.1						SILTSTONE	D	Par	Y	150	76	76	21.76	21.76	76.0	3.77	4.55	Undulating Rough	





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# Contract Number: 33821

Client's Reference: P16177 PO 9290

Laboratory Report

Report Date: 06-02-2017

Client Priority Geotechnical Limited Unit 12 Owenacurra Business Park Midleton Co. Cork.

Contract Title: Derreenacrinnin WF For the attention of: Colette Kelly

Date Received: **18-01-2017** Date Commenced: **18-01-2017** Date Completed: **06-02-2017** 

**Test Description** 

Youngs modulus & possions Ratio including UCS - @ Non Accredited Test

Magnesium sulfate test soundness value.

BS 812/Part BS EN 1367-2:1998 - * UKAS

#### Ten percent fines value (TFV)

BS 812-111:1990 - @ Non Accredited Test

**Disposal of Samples on Project** 

Notes: Observations and Interpretations are outside the UKAS Accreditation

- * denotes test included in laboratory scope of accreditation
- # denotes test carried out by approved contractor
- @ denotes non accredited tests

This certificate is issued in accordance with the accreditation requirements of the United Kingdom Accreditation Service. The results reported herein relate only to the material supplied to the laboratory. This certificate shall not be reproduced except in full, without the prior written approval of the laboratory.

#### Approved Signatories:

Alex Wynn (Associate Director) - Benjamin Sharp (Contracts Manager) - Emma Sharp (Office Manager) Paul Evans (Quality/Technical Manager) - Richard John (Advanced Testing Manager) - Vaughan Edwards (Managing Director)

GEO Site & Testing Services Ltd Unit 3-4, Heol Aur, Dafen Ind Estate, Dafen, Llanelli, Carmarthenshire SA14 8QN Tel: 01554 784040 Fax: 01554 784041 info@gstl.co.uk gstl.co.uk

#### Determination of Unconfined Compressive Strength. ISRM Suggested Methods Vol 16, No. 2, pp. 135-140 1979..

Date:	03-Feb-17
Contract Number:	33821
Client reference:	P16177
Location:	Derreenacrinnin WF
Sample Type:	Core
Sample Preparation:	Sawing and Grinding
Operator:	Jason Dawney

Borehole Number	Depth (m) from	Depth (m) to	Diameter (mm)	Length (mm)	Initial Mass (g)	Bulk Density (Mg/m3)	Dry Density (Mg/m3)	Moisture Content (%)	Load Failure (kn)	Maximum Compressive Strength (mpa)	Date Tested
BHT03	3.70		75.20	213.00	2567.8	2.71	2.71	0.20	72.1	16.2	31-Jan-17



#### For and behalf of GEO Site & Testing Services Limited

Paul Evans - Technical/Quality Manager Emma Sharp - Office Manager Ben Sharp - Contracts Manager Wayne Honey - Quality/Office Assistant



Date Approved:

3.2.17

23/08/2012

Unit 4, Heol Aur, Dafen, Llanelli SA14 8QN

GEO-RCK01



Client:	Priority G	eotechnical	Limited
Client Ref:	P16177		
Date Sampled:	unknown		
Date tested:	01/02/20	17	
Contract Number:	05/08/1992		
Location:	Derreenad	rinnin WF	
Hole Number	BHT01		
Sample Number:			
Depth (m):	5.35	to	6.45
Sample Type:	Core		
Method of Sampling:	n/a		
Sampled By:	unknown		
Test Condition:	Soaked		
Target Specification:	N/A		
Test Portion Size Fraction	10 – 14 m	m	
Mass of test Portions	420 –430	g	
Description			

Magnesium sulfate value (Test 1) 6 Magnesium Sulfate Value (MS) - % Magnesium sulfate value (Test 2) 7 **7** 

#### Remarks:

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#### For and behalf of GEO Site & Testing Services Ltd

Authorised By: Wayne Honey (Office/Quality Assistant)

W. Honey



Date:

6.2.17



Client: Client Ref: Date Sampled: Date tested: Contract Number: Location: Hole Number	Priority Geoted P16177 unknown 01/02/2017 05/08/1992 Derreenacrinn BHT02	hnical Limited
Sample Number:		
Depth (m):	5.7	to
Sample Type:	Core	
Method of Sampling:	n/a	
Sampled By:	unknown	
Test Condition:	Soaked	
Target Specification:	N/A	
Test Portion Size Fraction	10 – 14 mm	
Mass of test Portions	420 –430 g	
Description		

Magnesium sulfate value (Test 1) 9 Magnesium Sulfate Value (MS) - % Magnesium sulfate value (Test 2) 9 9

#### Remarks:

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#### For and behalf of GEO Site & Testing Services Ltd

Authorised By: Wayne Honey (Office/Quality Assistant)

W. Honey



Date:

6.2.17



Client:	Priority Ge	otechnica	l Limited	
Client Ref:	P16177			
Date Sampled:	unknown			
Date tested:	01/02/2017			
Contract Number:	05/08/1992			
Location:	Derreenaci	rinnin WF		
Hole Number	BHT03			
Sample Number:				
Depth (m):	5.9	to	6.8	
Sample Type:	Core			
Method of Sampling:	n/a			
Sampled By:	unknown			
Test Condition:	Soaked			
Target Specification:	N/A			
Test Portion Size Fraction	10 – 14 mr	n		
Mass of test Portions	420 –430 g	J		
Description				

Magnesium sulfate value (Test 1) 5 Magnesium Sulfate Value (MS) - % Magnesium sulfate value (Test 2) 6 6

#### Remarks:

This certificate is issued in accordance with the accreditation requirements of the United Kingdom Accreditation Service. The results reported herein relate only to the material supplied to the laboratory. This certificate shall not be reproduced in full, without the prior written approval of the laboratory.

#### For and behalf of GEO Site & Testing Services Ltd

Authorised By: Wayne Honey (Office/Quality Assistant)

W. Honey



Date: 6.2.17



Client:	Priority Geo	technica	l Limited	
Client Ref:	P16177			
Date Sampled:	unknown			
Date tested:	01/02/2017	7		
Contract Number:	05/08/1992			
Location:	Derreenacri	nnin WF		
Hole Number	BHT04			
Sample Number:				
Depth (m):	5.6	to	6.7	
Sample Type:	Core			
Method of Sampling:	n/a			
Sampled By:	unknown			
Test Condition:	Soaked			
Target Specification:	N/A			
Test Portion Size Fraction	10 – 14 mm			
Mass of test Portions	420 –430 g			
Description				

Magnesium sulfate value (Test 1) 7 Magnesium Sulfate Value (MS) - % Magnesium sulfate value (Test 2) 7 **7** 

#### Remarks:

This certificate is issued in accordance with the accreditation requirements of the United Kingdom Accreditation Service. The results reported herein relate only to the material supplied to the laboratory. This certificate shall not be reproduced in full, without the prior written approval of the laboratory.

#### For and behalf of GEO Site & Testing Services Ltd

Authorised By: Wayne Honey (Office/Quality Assistant)

W. Honey



Date:

6.2.17



BS 812 : Part 111 : 1990.

Client:	Priority Geotechnical Limited
Date Sampled:	unknown
Date tested:	02/02/2017
Contract Number:	33821
Location:	Derreenacrinnin WF
Sample Number:	BHT01
Depth (m):	1.65-6.45
Method of Sampling:	n/a
Sampled By:	unknown
Test Condition:	Soaked
Target Specification:	N/A
Test Portion Size Fraction	10 – 14 mm
Mass of test Portions	425 –430 g
Description	

The Average Ten Percent Fines value (kN) **TFV** :

110

**Remarks:** 

## For and behalf of GEO Site & Testing Services Ltd

Wayne Honey - Office/Quality Assistant Emma Williams - Office Manager

6.2.17





Date Approved:

BS 812 : Part 111 : 1990.

Client:	<b>Priority Geotechnical Limited</b>
Date Sampled:	unknown
Date tested:	02/02/2017
Contract Number:	33821
Location:	Derreenacrinnin WF
Sample Number:	BHT02
Depth (m):	1.55-7.00
Method of Sampling:	n/a
Sampled By:	unknown
Test Condition:	Soaked
Target Specification:	N/A
Test Portion Size Fraction	10 – 14 mm
Mass of test Portions	425 –430 g
Description	

The Average Ten Percent Fines value (kN) **TFV** :

117

**Remarks:** 

## For and behalf of GEO Site & Testing Services Ltd

Wayne Honey - Office/Quality Assistant Emma Williams - Office Manager





Date Approved:

6.2.17

BS 812 : Part 111 : 1990.

Client:	<b>Priority Geotechnical Limited</b>
Date Sampled:	unknown
Date tested:	02/02/2017
Contract Number:	33821
Location:	Derreenacrinnin WF
Sample Number:	BHT03
Depth (m):	1.55-6.8
Method of Sampling:	n/a
Sampled By:	unknown
Test Condition:	Soaked
Target Specification:	N/A
Test Portion Size Fraction	10 – 14 mm
Mass of test Portions	425 –430 g
Description	

The Average Ten Percent Fines value (kN) **TFV** :

109

**Remarks:** 

## For and behalf of GEO Site & Testing Services Ltd

Wayne Honey - Office/Quality Assistant Emma Williams - Office Manager





Date Approved:

6.2.17

23/08/2012

Unit 4, Heol Aur, Dafen, Llanelli SA14 8QN

GEO-AGG23

BS 812 : Part 111 : 1990.

Client:	Priority Geotechnical Limited
Date Sampled:	unknown
Date tested:	02/02/2017
Contract Number:	33821
Location:	Derreenacrinnin WF
Sample Number:	BHT04
Depth (m):	1.6-6.7
Method of Sampling:	n/a
Sampled By:	unknown
Test Condition:	Soaked
Target Specification:	N/A
Test Portion Size Fraction	10 – 14 mm
Mass of test Portions	425 –430 g
Description	

The Average Ten Percent Fines value (kN) **TFV** :

102

**Remarks:** 

GEO-AGG23

#### For and behalf of GEO Site & Testing Services Ltd

Wayne Honey - Office/Quality Assistant Emma Williams - Office Manager





Date Approved:

23/08/2012

6.2.17

Unit 4, Heol Aur, Dafen, Llanelli SA14 8QN

### APPENDIX D

## **EXPLORATION LOCATION PLANS & SURVEY DATA**

Exploration Location Layout

P16177-SI-A

Exploration Location Plans

P16177-SI-01 to P16177-SI-06

Exploratory Location	Easting	Northing	Elevation, mOD Malin	Final Depth, m bgl	Start date, dd/mm/yyyy
BHT01	510905.77	551801.88	375.64	7.10	28/11/2016
BHT02	511129.07	551861.03	395.70	7.00	25/11/2016
BHT03	511356.15	551903.01	398.85	6.80	25/11/2016
BHT04	511564.53	551957.93	384.06	6.70	23/11/2016
DP01	511386.11	549475.14	142.98	3.30	22/12/2016
DP02	511327.39	549769.63	145.55	1.90	22/12/2016
DP03	511264.03	550031.30	156.43	1.40	22/12/2016
DP04	511076.27	550454.73	174.95	1.20	22/12/2016
DP05	511028.37	550647.52	186.97	1.20	22/12/2016
DP06	510967.71	550739.18	191.63	1.10	22/12/2016
DP07	510868.00	550802.05	198.34	2.00	22/12/2016
DP08	510787.80	550844.01	202.37	0.60	22/12/2016
TP01	510809.58	550969.85	216.32	0.90	24/11/2016
TP01A	510882.03	551794.23	372.69	1.25	22/12/2016
TP02	510891.98	550989.76	225.10	2.10	24/11/2016
TP02A	511125.00	551856.90	396.25	0.70	22/12/2016
TP03	511062.64	551016.87	233.01	2.30	24/11/2016
TP03A	511344.22	551904.85	399.99	1.70	25/11/2016
TP04	511093.42	551120.27	235.97	1.70	24/11/2016
TP04A	511559.33	551956.84	384.52	1.05	25/11/2016
TP05	511197.23	551249.71	243.22	1.30	23/11/2016
TP05A	511798.49	552076.53	358.72	1.50	22/11/2016
TP06	511291.55	551357.60	256.13	1.20	23/11/2016
TP06A	511542.07	552109.44	377.39	0.80	24/11/2016
TP07	511382.47	551532.85	286.25	2.10	23/11/2016
TP07A	511760.44	552193.89	348.45	1.20	24/11/2016
TP08	511449.24	551596.66	293.53	1.60	23/11/2016
TP09	511594.68	551757.68	312.98	1.40	23/11/2016
TP10	511725.65	551895.24	329.09	1.90	23/11/2016
TP11	511851.30	552006.60	338.63	1.15	23/11/2016
TP12	511906.36	552114.95	349.14	2.00	23/11/2016
TP13	511670.28	552018.67	375.78	0.75	22/11/2016

Exploratory Location	Easting	Northing	Elevation, mOD Malin	Final Depth, m bgl	Start date, dd/mm/yyyy
TP14	511517.09	551958.43	385.00	1.30	25/11/2016
TP15	511623.30	552060.49	371.22	3.20	24/11/2016
TP16	511441.37	551948.82	396.17	0.65	25/11/2016
TP17	511360.97	551923.09	399.98	0.55	25/11/2016
TP18	511286.32	551894.88	398.18	1.60	25/11/2016
TP19	511041.42	551857.42	386.64	1.10	22/12/2016
TP20	510961.96	551859.11	377.72	1.40	22/12/2016
TP21	510813.20	551832.17	349.93	2.40	22/12/2016
TP22	510701.89	551995.91	311.55	0.95	22/12/2016
TPS1	511146.34	551192.30	239.83	2.10	22/12/2016
TPS2	511125.90	551212.43	243.24	2.20	22/12/2016
TPS3	511160.14	551231.59	242.44	1.90	22/12/2016



Priority Geotechnical Site
JOB NAME:
Derreenacrinnig Wind Farm Drimoleague
Sheet Title:
EXPLORATORY LOCATION LAYOUT
JOB NUMBER:
P16177
DRAWING NUMBER:
P16177-SI-A
DRAWN BY: Gary Curtin
DATE: 27/02/2017
SCALE: APPROVED: 1:10000 ON A3 GH
REVISION: D01
pgipriority geotechnical



**#** DP02

RET.			
TP00 Denotes Tri	al Pit location		
BH00 Denotes Bo	H00 Denotes Borehole location		
DP00 Denotes Dy	P00 Denotes Dynamic Probe location		
JOB NAME:			
Derreenacrinni	g Wind Farm		
Drimoleague			
Object Titles			
Sheet Title:			
EXPLORATIO	N LOCATION		
EXPLORATIO PLAN	N LOCATION		
EXPLORATION PLAN	N LOCATION		
EXPLORATION PLAN	N LOCATION		
EXPLORATION PLAN	N LOCATION		
EXPLORATION PLAN JOB NUMBER: P16177	N LOCATION		
EXPLORATION PLAN JOB NUMBER: P16177	N LOCATION		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER:	N LOCATION		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01	N LOCATION		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01	N LOCATION		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY:	N LOCATION		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY: Gary Curtin	N LOCATION		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY: Gary Curtin	N LOCATION		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY: Gary Curtin DATE: DATE:	N LOCATION		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY: Gary Curtin DATE: 27/02/2017			
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE:	APPROVED:		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3	APPROVED: GH		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 REVISION:	APPROVED: GH		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 REVISION: D01	APPROVED: GH		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 REVISION: D01	APPROVED: GH		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 REVISION: D01	APPROVED: GH		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 REVISION: D01	APPROVED: GH		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 REVISION: D01	APPROVED: GH		
EXPLORATION PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-01 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 REVISION: D01	APPROVED: GH		

## **#** DP03



KEY:			
TP00 Denotes Tr	ial Pit location		
BH00 Denotes Bo	Denotes Borehole location		
+ DP00 Denotes Dy	Denotes Dynamic Probe location		
JOB NAME:			
Derreenacrinni	ig Wind Farm		
Drimoleague	•		
Sheet Title:			
EXPLORATION LOCATION			
EXPLORATIO	N LOCATION		
EXPLORATIO PLAN	N LOCATION		
EXPLORATIO PLAN JOB NUMBER: P16177	N LOCATION		
EXPLORATIO PLAN JOB NUMBER: P16177	N LOCATION		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER:	N LOCATION		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02	N LOCATION		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02	N LOCATION		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02	N LOCATION		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02 DRAWN BY: Gary Curtin	N LOCATION		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02 DRAWN BY: Gary Curtin	N LOCATION		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02 DRAWN BY: Gary Curtin DATE:	N LOCATION		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02 DRAWN BY: Gary Curtin DATE: 27/02/2017	N LOCATION		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE:	APPROVED:		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3	APPROVED: GH		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 BEV(SION:	APPROVED: GH		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 REVISION: D01	APPROVED: GH		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 REVISION: D01	APPROVED: GH		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 REVISION: D01	APPROVED: GH		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 REVISION: D01	APPROVED: GH		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 REVISION: D01	APPROVED: GH		
EXPLORATIO PLAN JOB NUMBER: P16177 DRAWING NUMBER: P16177-SI-02 DRAWN BY: Gary Curtin DATE: 27/02/2017 SCALE: 1:2,000 ON A3 REVISION: D01	APPROVED: GH		



**#** DP05

KEY: TP00 BH00 TP00	Denotes Tria	al Pit location rehole location namic Probe location
JOB NAME: Derreer Drimole Sheet Title: EXPLO PLAN	nacrinni ague RATION	g Wind Farm
DRAWING N P16177 DRAWING N P16177 DATE: 27/02/20' SCALE: 1:2,000 C REVISION: D01	UMBER: 2-SI-03 tin 17 DN A3	APPROVED: GH




	KEY:
	TP00 Denotes Trial Pit location
	BH00 Denotes Borehole location
	DP00 Denotes Dynamic Probe location
TP03A 🖬 🕘	
BHT03	
	JOB NAME:
	Derreenacrinnig Wind Farm
	Drimoleague
	Sheet Title:
	FLAN
	JOB NOWBER.
	P16177
	DRAWING NUMBER:
	P16177-SI-05
	DRAWN BY:
	Gary Curtin
	DATE
	27/02/2017
	21/02/2017
	SCALE: APPROVED:
	1.2.000 ON A3 GH
	,
	REVISION:
	D01
	Annonementariante
Ν	
	<b>     </b>   prioritu
	geotechnical







## D01

1:2,000 ON A3

APPROVED: GH

27/02/2017

DATE:

SCALE:

REVISION:

Gary Curtin

DRAWN BY:

P16177-SI-06

DRAWING NUMBER:

P16177

JOB NUMBER:

PLAN

Sheet Title: EXPLORATION LOCATION

KEY:

TP00 Denotes Trial Pit location BH00 Denotes Borehole location + DP00 Denotes Dynamic Probe location

## Derreenacrinnig Wind Farm Drimoleague

JOB NAME:

APPENDIX E

CALIFORNIA BEARING RATIO, CBR ASSESSMENT

Summary of California bearing ratio, CBR data									
Location	CBR, % (unadjusted)	CBR, % (adjusted)	CBR, % (Lab)	Depth, m bgl	Stratum	Capping thickness, mm	Sub- base, mm	w/PL	
Turbines									
TP01A	22	11		0.4	Slightly sandy slightly gravelly SILT	200	150		
TP02A	22	11		0.2	Slightly sandy slightly gravelly SILT	200	150		
TP03A	8	4	1	0.6	Slightly gravelly sandy SILT	300	150		
TP04A	2	1		0.2	Slightly sandy slightly gravelly SILT	600	150		
TP05A	9	5	1	0.8	Slightly sandy gravelly SILT	250	150		
TP06A	5	3		0.6	Slightly sandy slightly gravelly SILT	600	150		
TP07A	9	5		0.6	Slightly sandy gravelly SILT	250	150		

Summary of California bearing ratio, CBR data									
Location	CBR, % (unadjusted)	CBR, % (adjusted)	CBR, % (Lab)	Depth, m bgl	Stratum	Capping thickness, mm	Sub- base, mm	w/PL	
Sub-station									
TPS1	9	5	3	0.4	Silty sandy GRAVEL	350	150		
TPS2	9	5		0.4	Slightly sandy gravelly SILT	250	150		
TPS3	5	3	2	0.9	Slightly sandy gravelly SILT	600	150	1.0	

Summary of California bearing ratio, CBR data										
Location	CBR, % (unadjusted)	CBR, % (adjusted)	CBR, % (Lab)	Depth, m bgl	Stratum	Capping thickness, mm	Sub- base, mm	w/PL		
	Access road									
TP01	9	5		0.75	Silty sandy GRAVEL	250	150			
TP02	22	11	2	0.6	Very silty very sandy GRAVEL	200	150			
TP03	9	5		0.8	Slightly sandy gravelly SILT	250	150			
TP04	3	2	2	1.0	Slightly sandy gravelly SILT	600	150	1.1		
TP05	5	3		1.0	Slightly sandy gravelly SILT	600	150	1.5		
TP06	9	5	3	0.8	Slightly sandy slightly gravelly SILT	350	150			
TP07	7	4		1.3	Slightly gravelly sandy SILT	600	150	1.2		
TP08	5	3	1	0.9	Slightly sandy gravelly SILT	600	150	2.2		
TP09	22	11		0.4	Slightly sandy slightly gravelly SILT	200	150			
TP10	5	3	1	0.9	Slightly sandy gravelly SILT	600	150			
TP11	5	3		1.3	Slightly sandy slightly gravelly SILT	600	150	1.2		
TP12	0	0		2.3	Clayey PEAT	600 **	150	w = 745%		
TP13	9	5		0.7	Slightly gravelly sandy SILT	250	150			
TP14	9	5	1	0.8	Slightly sandy slightly gravelly SILT	250	150			
TP15	5	3		1.2	Slightly sandy gravelly SILT	350	150			
TP16	22	11		0.45	Slightly sandy gravelly SILT	200	150			
TP17	22	11		0.45	Slightly sandy slightly gravelly SILT	200	150			
TP18	22	11		0.6	Slightly sandy gravelly SILT	200	150			
TP19	4	2	1	0.9	Slightly sandy gravelly SILT	600	150	1.5		
TP20	16	8		0.45	Slightly sandy gravelly SILT	200	150			
TP21	9	5		0.6	Slightly gravelly sandy SILT	250	150			
TP22	9	5	1	0.9	Slightly gravelly sandy SILT	600	150			